

**INFORMATIONAL MODEL OF THE SUBJECT OF PLEASING
ACCUMULATED INFORMATIVE SOURCE**

By

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NOTATION

English Symbols

L	characteristic length
d	settlement width
δ	gap
A_p	projected flow area of + ridges = $A_p A_g$
A_g	specific flow area = A_g / A_p
A_s	net area as a function of time
b	ridgeless channel width
R_d	dimensionless friction coefficient
C	dry discharge coefficient $1/\sqrt{R_d^{1/2}}$
C_d	dry resistance
C_1	experimental proportionality constant $1000 A_p A_g C_d$
C_1, C_2	experimental constants for flow to water-saturated channel
ϵ	deviation of actual value from expected value
E_a	actual expected net computation value = ϵ
F_d	drag factor
H_p	ridges elevation

D_p	variable used to determine particle drug distribution
L	minimum operating length of ring shear chamber
N	Reynolds number
P_f	Friction Factor for flowline over a bed of particles
P_{fr}	sliding friction constant term
P_s	shear force
P_w	inertia
R	acceleration due to gravity
Re	newt flow rate
S	ring shear chamber height
T	moment of inertia
V_p	height of gate for epoxides, and ring shear
W	shear resistance
Δ	flow length
δ	smaller layer length
l_p	distance length
L	bed shear layer length
L'	variable used to determine particle drug distribution
N	Reynolds number exponent
η	viscosity
n_p	total mass of particle in a unit
σ	Shearling's ring shear coefficient
θ	constant sample size
θ_p	bottom oil particle in a unit
τ	number of major contributions accounted by flow through a bed of epoxides

ρ	applied load
η	undrained shear wave deformation of granular media
r	optical radius
R	hydraulic radius
R_s	standard error of residuals
R_p	standard error of surfaces
γ	deformation of discretized sub-section of a specimen
$\dot{\gamma}$	velocity of body rotation in unyielded fluid
V_0	friction velocity = $\sqrt{w_0 g}$
α	normal approach of a flat plane to the center of a cylinder or rod
β	longitudinal spacing of transverse builds
δ	spatial displacement
δ	height of surface between a cylinder and a flat plate
P_0	mean depth of flow

Greek Symbols

δ	displacement or deflection
η	intrinsic viscosity
ρ	density
τ_{ij}	shearing stress at i,j
ψ	Volume's ratio
π_0	external pressure

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**HYDROLOGICAL MODEL OF THE EROSION OF FLUENT
ASSOCIATED WITH HISTOSOL SOILS**

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In this study, examples of specific plant communities were analyzed to find more efficient barriers could be designed. Fluvial investigated here under irrigation (*Fluticaria-spatulata*) with no flowing hydrosites did give signs to form more pr aggregates because unbroken, interacting to separate and then connecting. They are currently harvested to control their propagation to ensure earlier only there is chemical or biological method to prohibited or unsuccessful. More efficient harvesting methods will depend on multiple designs based on hydrological characteristics of soils, which up to now have been inadequately used to predict performance of testing or removal systems.

Empirical models were developed that describe multiple user kinematic information for six different harvesting operations (rectangular bid, leveling and forward suspension). The mathematical description of user was based on experimental observations and physical and biomechanical principles. Primary system variables are horizontal displacement and velocity of the user, and other input variable is either leveling or suspension velocity at a function of time.

Equations derived from the empirical models were solved numerically, and output variables such as suspending and leveling forces, user kinematics, and change in net length were predicted over time with good correlation with experimental data. The slope coefficients were found to be independent of harvester model but depending on net geometry. Power inputs to harvester can be reduced by careful selection of suspension patterns, leveling or suspending velocity, net geometry and existing plant densities.

CHAPTER 1 (CONTINUED)

Water hyacinth (*Eichornia crassipes*) are free-floating, emergent hydrophytes or, in marshes, emergent sedges that are positively buoyant. These plants grow together to form a one or two-metre tall, dense and continuous emergent system. In Florida water hyacinths were eighth in acreage of all aquatic plants surveyed in 1990, and they are poor biomass cropping materials (Blaauw, 1990). Florida's Surface Water Improvement and Management Act (SWIM) includes aquatic plant management strategies in certain water bodies. Additional restrictions on pesticide use by the Environmental Protection Agency and the U.S. Fish and Wildlife Service still now apply to herbicides used within ranges of protected plants and animals (Dame, 1987). These political issues and historical use of water hyacinths in water treatment systems drive the development of effluent control technologies to manage aquatic plants without the use of heavy, expensive mechanical equipment or permitted by dredging canals and reduce the time delay often found in biological control methods.

The current cost of mechanically removing water hyacinths in South Africa dollars per acre is (Poggenpohl et al., 1997). Typically, 200

two of plants are found in one hectare; consequently the cost to harvest one hectare is \$800 to \$1000. On the other hand, chemical treatment costs approximately \$500/ha. The U.S. Army Corps of Engineers considers a system which harvests and dries out 80 to 100 tons per hectare to be efficient enough to harvest. Some growth rates of the major forests (Klopper and Senn, 1979) The Corps also found that the more advanced harvesting system available can not meet these conditions, and that the principal problem is the transportation of the plants from the sites. For instance, one way of transportation is trucking a cut-off plants. This, however, should not be used. The plants tend to dry. Rapp (1960) described the mechanism of jet formation as a combination of separation of plants and a resulting acceleration that increases the vertical dimension of the air. As a consequence of a jet, living trees become, and immediately start to die under the continuous direct and impact.

An additional method however, does depend on a higher dryness than is based on hydrodynamic characteristics. (e.g. dry stems and the elastic properties) of some lignocellulose however physical characterization of specific plants has been insufficient to predict the performance of living trees in removal systems. The purpose of this research are to determine hydrodynamic properties of some lignocellulose necessary to predict the response of a tree; lignocellulose and its working force on velocity input and boundary conditions.

CHAPTER 11 STRUCTURE

The principle objectives of this research are listed below and each objective will be the focus of a subsequent chapter.

- Quantitatively describe those physical characteristics of water flow which distinguish one type of aggregation from another.
- Determine the hydrodynamic and elastic contact properties of water flocs/flocs and find the equations that describe the relationships between viscous forces on flocs and contact velocity and between contact forces and plane displacement within the floc.
- Develop, from developed constitutive relations and structural equations, a mathematical model that predicts how a particular suspension of water flocs/flocs reacts to an applied velocity.
- Validate the model and demonstrate potential applications for it.

CHAPTER III PHYSICAL CHARACTERISTICS OF WATER HYACINTH PLANTS

Several physical characteristics of individual water hyacinth plants and of different species have been described in this chapter—standing density, plant mass and biomass, net buoyancy and compressibility. The latter is a new property. Generally, plant characteristics vary according to habitat, season and plant age, and among different species.

Plant.—The morphological description of water hyacinth provides expected ranges of plant mass, leaf, root, and rhizome length and diameter and the internal cellular structure of the plant. This information is useful for the determination of biomass (e.g., plant mass) and for the identification of the plant parts that physically interact with man or with other plants.

The shapes of rhizomes and roots, the principal plant parts projecting into the water, influence the various types of growths that the plants can form. An empirical method to predict biomass length from other physical characteristics would be useful. Rhizomes are difficult to measure because they are usually heavily covered with roots and debris.

floating biomass density, its relationship with increasing speed and wind, describes the capacity of a benthos and the poor importance for managing or transposed species. Also, plant mass affects (spatial) forces measured in a benthic equation. On the other hand, area population density is useful to fine scale because total preferences in flow depends on the number of plants. The assessment of area population density is also important because it requires hardly every plant; yet, an empirical method to predict population density from other physical descriptors (e.g. plant mass or length) seems unclear.

Net primary: the fluxes required to subsist a given area of water benthos. Is useful for the study of energetic losses and recruitment processes, when the plants under certain conditions full under such devices. River benthic plants can be measured through leaf enclosures and sieves. These have been characterized by either stability, reproduction or biomass density and, as a related issue, by biogeo. For the literature reports on primary they might quantify the "connectivity" of a site; therefore, a tool described in this chapter can be used to measure net connectivity. Biomass is necessarily to control what is not to be expected with smaller and in the filling or lifting from the water. More information on connectivity changes could be useful in biogeographic studies. Tidal movement is responsible to a net small biomass but the plants have been subjected to physiological stresses.

The objectives of this chapter are:

- trace free available biomass the migration and composition of river benthos

- Determine the distribution of mass in a given species' population and how that distribution would vary from one population to another.
- Develop a relation between seedling density and average plant size of a tree.
- Develop an analytical method or technique estimate length.
- Determine how tree frequency varies for different sizes, and that is related with the size of plants when observation is to predict it.
- Determine consistency for different tree types.

Review of literature.

Reproductive Potential of the Rosewood.

The entire rosewood plant is a tree-climbing (semileg) emergent halophyte that consists of roots, rhizomes, stolons, leaves, inflorescences and fruit clusters (Figure 3-2). In flooding wetland conditions, along the river banks in the range of 1-3 m across and 80% of the annual plant dry weight is in the leaves and petioles. In non-flooded conditions, along the river banks are in the range of 4-5, and up to 90% of the dry weight is in the foliage (Baldy, 1988). Rooted-stemmed water hyacinths (100-200 g/m² dry) are harvested from sewage treatment plants because plant densities are optimal for harvesting without harming plants (Gledhill, 1988).

The fibrous root (as opposed to a tap root) has a dense cluster of main and lateral roots. The lateral roots are not randomly dispersed, and they are arranged in groups along the main root axes (Ballalay et al., 1999). The roots of plants from different habitats and seasons may differ in diameter but can vary greatly in length. The diameter varies from 0.4 to 1.0 mm. The length varies from 4 to

Figure 2.12 A scatter histogram comparison of Deemed, Actual, Estimated, and Average.



from 10 small plates to 30 on 10 surface plates, and 10 to 30 in 10 large plates (Prestwich and Bailey, 1966; and Basson et al., 1982).

A third tree division region contains cortex and pith. The epidermal cells are either oblong, square or rhomboidal in shape and are 10 to 40 μ m in size. The cortical region consists of an outer layer of parenchyma cells, a cortical tissue that makes 75% of the cortex, an inner layer of parenchyma cells and an endodermis that measures 25% of the cortex. The pith consists of a pith region, 10 to 15 layers of parenchyma cells, and 10 to 15 cells of phloem (Basson et al., 1982).

A rhizome is a stem or root with short internodes. The rhizome produces all the biomass under all the roots, leaves, tubers, rhizomes and rafflyngeneses of the plant. The diameter of the rhizome has from 3 to 8 mm and the length from 1 to 30 m. Structurally similar to a root, a rhizome consists of epidermal, cortical and pith regions. The pith contains very few fibers, but some tracheary elements are present which increase in number and size as the rhizome extends. The cortical region consists of parenchyma tissue and air spaces (Basson et al., 1982).

A leaf consists of a petiole or blade (in small plants or trees living in very open areas) a blade and petiole or lamina (between petiole between the blade and the blade). Leaves are located to the petiole. They are round or pear-shaped and range from 3 to 4 cm in diameter. They are formed when the physiologically availability of water in the plant is great (Bar, 1999). Leaves, in very arid areas of other species, leaves are usually absent. Petiole length increases with increasing plant growing density (Odeier, 1981). In a

$\text{km}^2 \cdot \text{yr}^{-2}$) - probably length was 20 cm or $10 \text{ kg} \cdot \text{m}^{-2}$ (m), length was 15 and or $90 \text{ kg} \cdot \text{m}^{-2}$ (m), length was 30 cm. Generally, the three types of leaves are oriented in a nearly horizontal position (30 to 45° from horizontally) whereas the upright leaves (e.g. leaves where bases overlap within or above one) approach verticality (75° from the horizontal) (Preston and Darby, 1990). In dense stands, most *Lysimachia* leaves grow upright rather than horizontally over the soil surface (Darby, 1989).

Leaves originate from a rhizome and produce no offshoots or vegetative or floral structures. Leaves can be relatively short and vertical in dense stands but long and horizontal in open positions. They are 1 to 15 cm in diameter and 4 to 20 cm long in open stands or 8 to 15 cm long in close stands (Preston and Darby, 1990 and Darby et al., 1992).

Internal structures of various organs of the aerial part of a plant are related to each other (Gasser et al., 1990) and they can be classified into three groups: parenchymatous cells, support and conducting cells, and cell spaces. Ninety-five percent of the tissues of a plant is parenchymatous or consists of unspecialized cells that usually have well-defined cellular structures and they are used optimally for storage of proteins, starch grains, oil, etc., and the chrysophylloids cells in the blades.

Cellular vascular function is addition of phloem and xylem vessels, are scattered throughout the aerial part of a plant. Phloem functions in conduction of food and xylem functions in conduction of water and minerals and to support. Besides vascular function, fiber and tracheids, which are types of xylem providing mechanical support

are randomly placed in a plant. Unorganized fibers are subject to blisters, cracks and petioles, but rare in raphides and rhizomes while tracheids can be found throughout in mature plants. The fibers are 1.5 to 3 m long by 2 to 12 μ in diameter, whereas tracheids are elongated and twisted in shape and can be stretched far before they break.

The system allowing water transpiration to fluctuate and limited to shallow intercalating spaces; air chambers and pores along walls. Air chambers or lacunae are periodically characterized by pores (holes) and they appear as narrow passages always failing over to have been pulled apart at the corners by the continuous growth of the epidermis. The air chambers permit diffusion of gases and equalized pressure. The main portions of the air chambers, lacunae or diaphragms present floating because these structures are performed with silicon saprophytic only for the passage of gases. The system of lacunae is continuous from leaves to fruits.

Chemical Composition.

Basically, the main lignified consists of water. Deacon and Bartsch (1980) and Bunting et al. (1971) determined that 60 to 90% of the fresh content of roots, 50-58 of rhizomes, 50-58 of stolons, 50-58 of stems, and 57 to 69-72 of blades in water.

Depending on the existing diversity of plants, 50 to 90% of the dry weight of a plant is carbon (in leaves, stem, rhizome and rhizoids), around 10% in carboxylic acids and 1-2 to 2% in nitrogen. (Deacon and Bunting, 1971)

Density and Biomass.

The densities of various parts of plants were determined by the volume weights method by Pechoux and Berlese (1960). It was found that the specific weight of a root was 0.365, of a rhizome 0.825, of a stolon 0.818, of a flower 0.138 and of a leaflet 0.741.

Pechoux and Berlese (1960) also observed the limiting capacity defined as the weight required to submerge a set of plants, $wL = 1.5 \times 10^3$ mg of roots or 0.117 to 0.225 mg^{-2} (m). The latter corresponded to a population density of 200 plants/m².

Botanical and Physiological.

Botanical literature exists concerning growth rates, morphology and chemical composition of water hyacinths. Qualitative information is used to establish criteria that could be used to physically distinguish different types of water hyacinth plants. This section presents the procedures that were performed to determine characteristics that physically defined water hyacinth types.

Seedling Stage and Plant Characterization.

Total plant length, rootlet length, root length, longest leaf and plant leaflet width, plant mass and blade mass were recorded as plant characteristics.

The sample size of plants was chosen by this committee and undertaken based on sample variance, confidence interval and confidence level (Steel and Torrie, 1960). Choice of an adequate sample size to determine certain physical characteristics of a crop was complicated because each type of measurement had associated to it a different variance. In other words, sampling was from a heterogeneous distribution.

For example, calculations showed that within one year, plant mass was much more variable than plant length. Since herbarium primarily represents through sections, herbarium plants of many sizes are expected in a given area. Indeed, a sample standard deviation of plant mass calculated from a sample size of seven was only half of the average plant mass. Calculations based on this standard deviation indicated that a sample of 400 plants would be needed for a sample mean of plant mass to fall within 5% of the population mean with confidence 0.95. On the other hand, calculations based on an observed sample standard deviation of plant length showed that only 16 or 36 plants would be needed to obtain a population plant length within 5% of the true mean length and with 0.95 confidence. To obtain a population plant length within 10%, a sample of 8 to 10 plants would be needed.

In this study, except when otherwise noted, seven plants were determined for age and component description per site by herbarium. Seven plants permitted an accurate approximation for mean plant length; and the sites of fully characterized *s* were prioritized.

Plant Mass Distribution and Biomass Length.

The total plant mass relative to positive biomass length from other plant descriptions was simple, and a distribution of plant mass based on mass was compared to positive plant site biomass of all other population groups.

Plants from the University of Florida Herbarium (Hort), Bok Tower, Lake Wales, and FGCU Bellwood Fl. Field Laboratory were characterized according to the general procedure described in the

previous sections except that more than one plant was sampled from each site for which number and biomass were measured. Data from these observations were compiled to indicate that this represents different plant size and conditions. The plants from the two sites and from different field long rows. The plants from these 2 sites were very heterogeneous and no types were separated persistently, and they had short stems.

The classification of plant size within sites was attempted by this method. Thus, sample populations of plants were measured to determine if the sites that we observed could be divided into 3 (*i.e.* clusters and subgroups). Once this was determined, the mean values were attributed to plant size classes and their frequency noted. A histogram for each sample population of these frequencies was plotted using the chi-square test to determine if the distribution of the size classes within each site was normal.

Connectivity, Reserves, and Area Distribution.

After a physical analysis of these biological sites, it was observed that sites could differ by average size of plants, standing density, and frequency and connectivity. Except for connectivity, there were established methods to measure all of the mentioned characteristics; therefore, it failed to measure connectivity was desired. Connectivity is a measure of the degree of management through perennials and annuals. Here connectivity is implied in areas where plants are not touching each other, and the plants are not associated by roots. The principle of the derived connectivity rate is that no connectivity reserves, were significant nor were the required to influence a man-

Coverability, frequency, and seedling density were measured using the following technique: a 0.75 m^2 quadrat was placed down, weighed and a scale. Measurements were performed as follows:

A small steel frame was placed on top of undisturbed soil. Brackets were placed on the frame and it was bent under the soil. The weights were removed. Frame and soil were cut along the corresponding planes, and weights were added again. After such test, plants were disconnected, and all the plants within the frame were weighed and counted. The test was repeated several times at the same site in different types of plant mass and rates at different soil frequency and calculated as percentage to calculate the non-disconnected plants over the area of the frame (P_d) and connected as the difference between the percentage to subserve the connected and disconnected plants (P_c). Now site had similar frequency values but different coverabilities. To determine mostly the effect of coverability on seed availability, the ratio of coverability to soil frequency were calculated for every site tested, and this value was termed coverability ratio (D). The new frame square relation of germinating population density and frequency from other site disconnected were sought.

Density and Disconnection.

Cover, Frequency Density, Disconnection.

All test site of plant mass data were for the sites accepted had overall disconnection. However, some values were different. Disconnection values ranged from -0.2 ± 0.0 to -0.2 ± 0.4 for the germinating populations. Disconnection is 0.0 ± 0.0 for the ungerminated ones. A disconnection value of zero signified a small population

the standard deviation is given by the square root of σ^2 divided by the number of samples (Pearce et al., 1990).

Plant size was represented by size classes and various sub-classes (Table 3.9). This classification system may have to be expanded by the addition of other sub-classes if PII other population groups are sampled. Class 0 represents plants that weigh less than 0.1 kg. These plants are subdivided into sub-classes, because the largest and smallest plants in a given population will often only the adjoining classes (e.g. the smallest plants fall in class 0 and the largest in 1). Class 6 represents large plants that weigh over 0.9 kg. The class structure follows a linear increase in mass until class 5. Classes 6 through 9 allow for greater mass differences, because larger and older plants are structurally different from younger and smaller plants. Larger plants usually have more leaves and lower mortality, and they have part of their mass in developing leaves that eventually slough off.

Table 3.9 Plant size classes
Sub-classes

Class	Plant mass kg	Plant mass kg
0	0	0.00-0.09
	0	0.09-0.18
	0	0.18-0.27
	0	0.27-0.36
	0	0.36-0.45
	0	0.45-0.54
	0	0.54-0.63
	0	0.63-0.72
	0	0.72-0.81
	0	0.81-0.90
1	1.0	0.10-0.13
	1.2	0.13-0.17
	1.7	0.17-0.20
2	2.0	0.20-0.30
3	3.0	0.30-0.50
4	4.0	0.50-0.70
5	5.0	> 0.70

Histograms constructed with these clusters were normally distributed (Figures 3.1. - 3.3), and these may distributions came from well-established population groups. These averages varied according to size of village or location. After the data from the non-subsistence group (Figure 3.4) was omitted, it was determined that the populations could be represented by more than one group of places, one dominated by class 0 places and the others by higher classes (Figure 3.5). This group of places was not well established because their populations were concentrated for regular markets. The histograms illustrate that major landmarks may occur greatly earlier than the locations in another and even within the same location if the populations in a place are well established.

House Length.

Data on distance length from all locations were aggregated and graphed together. Distance length is linearly correlated to place weight (Figure 3.6). Length varied from 1 to 31 centimeters. Two linear regression equations that relate distance length to place were established; one for places that showed considerable overall change or were concentrated in the richness and one for locality places with single village.

House Density.

Human density in this study varied from 0 to 20 people² (m²), and population density varied from 0 to 300 places². However, no apparent relationship exists between the two types of density (Figure 3.7); the variance of population density is the quotient of the human density (people²) at a given aggregation level (i.e. average place mass (average of 7 places)). This simple relation does not well fit

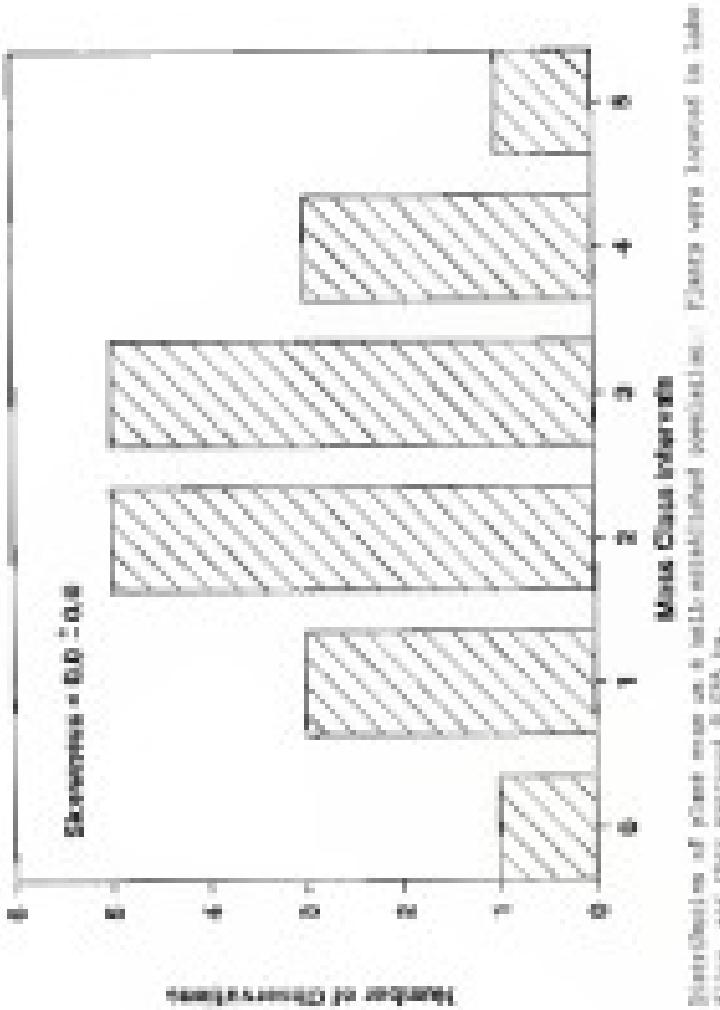


FIGURE 1. Distribution of plant species at two wave exposure levels. Plants were located in 100 cm² plots, and their height is 100 cm.

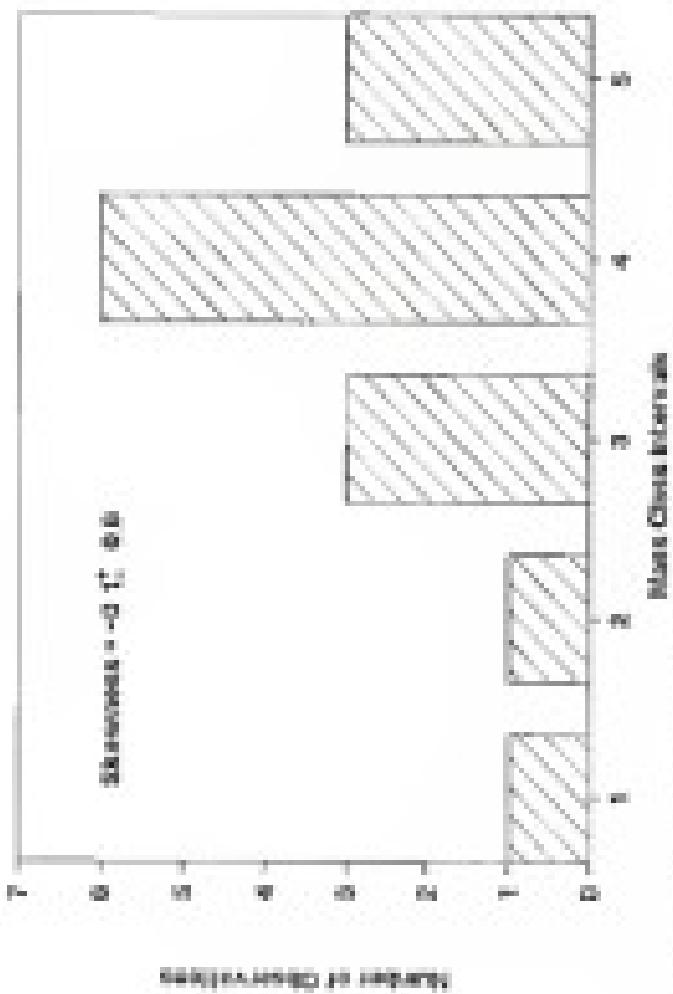
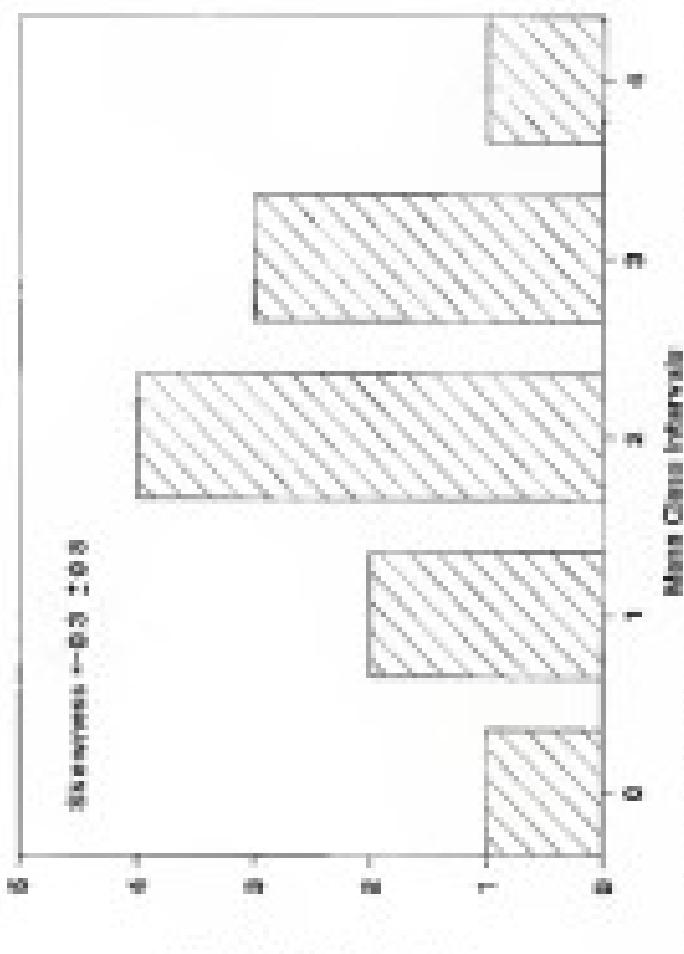


Figure 3.1 Construction of Pythagorean quadrilaterals. The sides were labeled as Hypotenuse = 1, and the areas are shaded.

Figure 3.2. Impact of different parameters on the probability of finding a target. The parameter values used in this figure are: $\alpha = 0.05$, $\beta = 0.05$, $\gamma = 0.05$, $\delta = 0.05$, $\epsilon = 0.05$, $\eta = 0.05$, $\theta = 0.05$, $\phi = 0.05$, $\psi = 0.05$, $\zeta = 0.05$, $\kappa = 0.05$, $\lambda = 0.05$, $\mu = 0.05$, $\nu = 0.05$, $\rho = 0.05$, $\sigma = 0.05$, $\tau = 0.05$, $\omega = 0.05$, $\chi = 0.05$, $\psi_1 = 0.05$, $\psi_2 = 0.05$, $\psi_3 = 0.05$, $\psi_4 = 0.05$, $\psi_5 = 0.05$, $\psi_6 = 0.05$, $\psi_7 = 0.05$, $\psi_8 = 0.05$, $\psi_9 = 0.05$, $\psi_{10} = 0.05$, $\psi_{11} = 0.05$, $\psi_{12} = 0.05$, $\psi_{13} = 0.05$, $\psi_{14} = 0.05$, $\psi_{15} = 0.05$, $\psi_{16} = 0.05$, $\psi_{17} = 0.05$, $\psi_{18} = 0.05$, $\psi_{19} = 0.05$, $\psi_{20} = 0.05$, $\psi_{21} = 0.05$, $\psi_{22} = 0.05$, $\psi_{23} = 0.05$, $\psi_{24} = 0.05$, $\psi_{25} = 0.05$, $\psi_{26} = 0.05$, $\psi_{27} = 0.05$, $\psi_{28} = 0.05$, $\psi_{29} = 0.05$, $\psi_{30} = 0.05$, $\psi_{31} = 0.05$, $\psi_{32} = 0.05$, $\psi_{33} = 0.05$, $\psi_{34} = 0.05$, $\psi_{35} = 0.05$, $\psi_{36} = 0.05$, $\psi_{37} = 0.05$, $\psi_{38} = 0.05$, $\psi_{39} = 0.05$, $\psi_{40} = 0.05$, $\psi_{41} = 0.05$, $\psi_{42} = 0.05$, $\psi_{43} = 0.05$, $\psi_{44} = 0.05$, $\psi_{45} = 0.05$, $\psi_{46} = 0.05$, $\psi_{47} = 0.05$, $\psi_{48} = 0.05$, $\psi_{49} = 0.05$, $\psi_{50} = 0.05$, $\psi_{51} = 0.05$, $\psi_{52} = 0.05$, $\psi_{53} = 0.05$, $\psi_{54} = 0.05$, $\psi_{55} = 0.05$, $\psi_{56} = 0.05$, $\psi_{57} = 0.05$, $\psi_{58} = 0.05$, $\psi_{59} = 0.05$, $\psi_{60} = 0.05$, $\psi_{61} = 0.05$, $\psi_{62} = 0.05$, $\psi_{63} = 0.05$, $\psi_{64} = 0.05$, $\psi_{65} = 0.05$, $\psi_{66} = 0.05$, $\psi_{67} = 0.05$, $\psi_{68} = 0.05$, $\psi_{69} = 0.05$, $\psi_{70} = 0.05$, $\psi_{71} = 0.05$, $\psi_{72} = 0.05$, $\psi_{73} = 0.05$, $\psi_{74} = 0.05$, $\psi_{75} = 0.05$, $\psi_{76} = 0.05$, $\psi_{77} = 0.05$, $\psi_{78} = 0.05$, $\psi_{79} = 0.05$, $\psi_{80} = 0.05$, $\psi_{81} = 0.05$, $\psi_{82} = 0.05$, $\psi_{83} = 0.05$, $\psi_{84} = 0.05$, $\psi_{85} = 0.05$, $\psi_{86} = 0.05$, $\psi_{87} = 0.05$, $\psi_{88} = 0.05$, $\psi_{89} = 0.05$, $\psi_{90} = 0.05$, $\psi_{91} = 0.05$, $\psi_{92} = 0.05$, $\psi_{93} = 0.05$, $\psi_{94} = 0.05$, $\psi_{95} = 0.05$, $\psi_{96} = 0.05$, $\psi_{97} = 0.05$, $\psi_{98} = 0.05$, $\psi_{99} = 0.05$, $\psi_{100} = 0.05$.



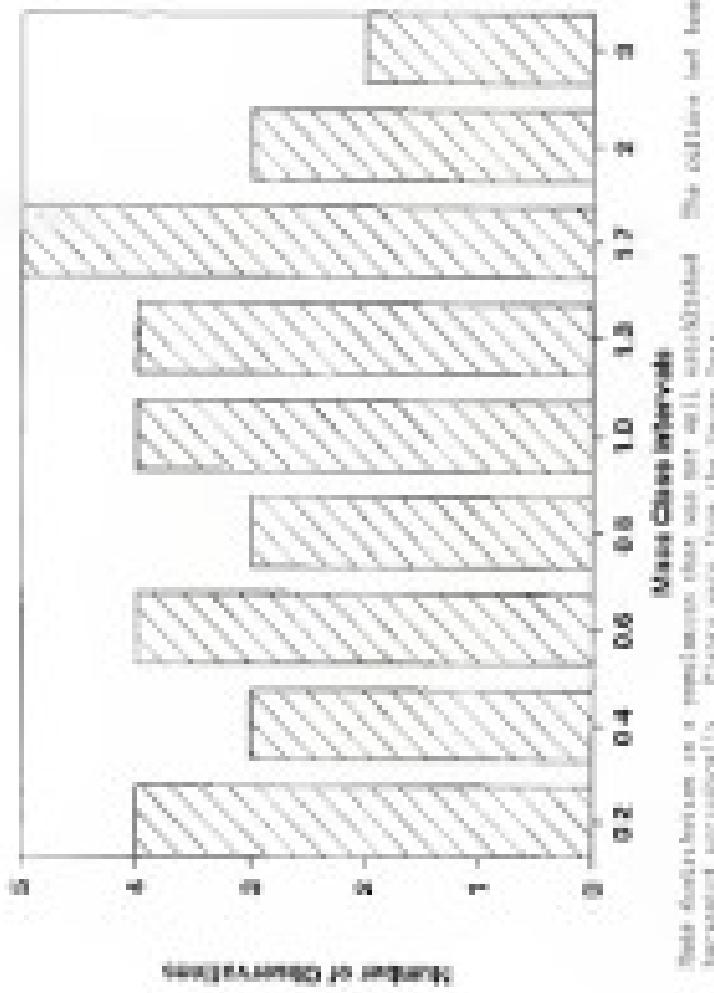
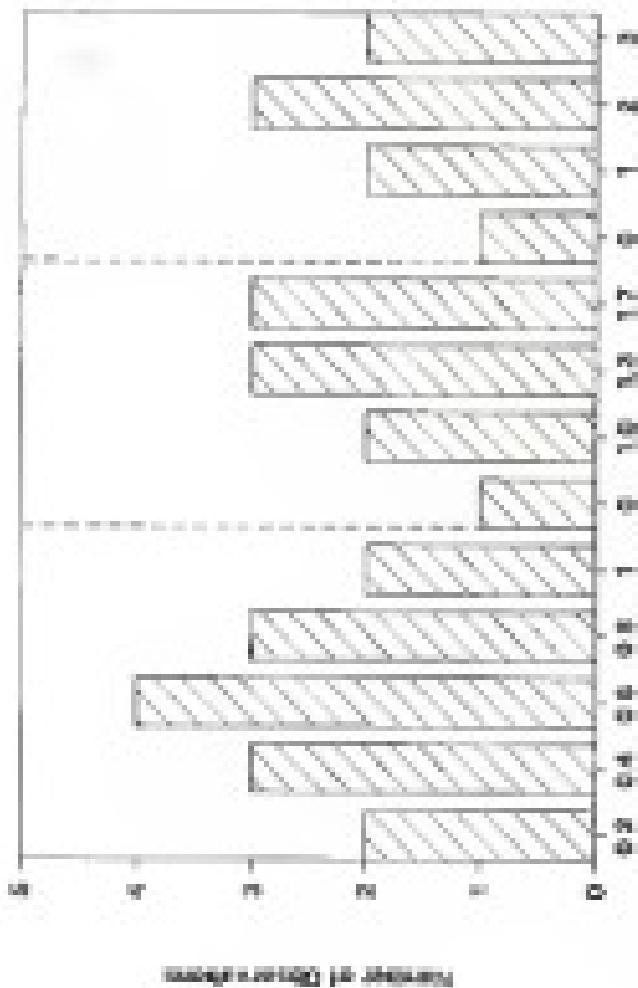


Figure 1. User confidence levels in a visual search task were measured in two different participants (15 pictures each from the same set).

Figure 7.3. Figure 7.2(a) restructured to show some of the features of the model. The figure consists of four panels, each showing a different time step. The horizontal axis is time, and the vertical axis is space. The values in the grid are represented by diagonal hatching.



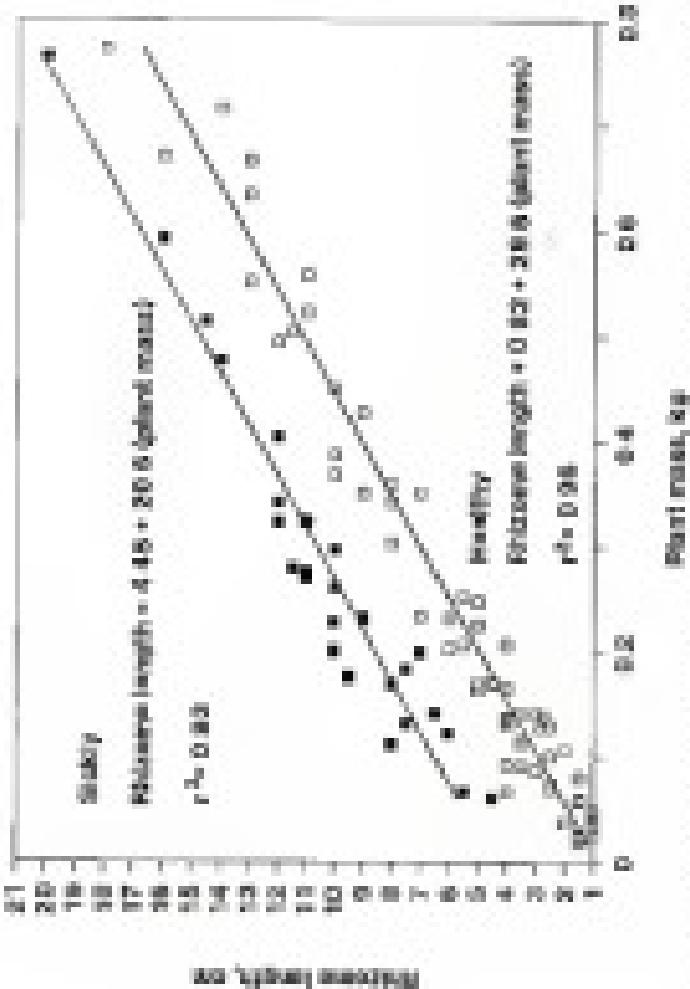


Figure 1.4. Shoot length is correlated to plant height. Data of 26 rice plants at different plant heights.

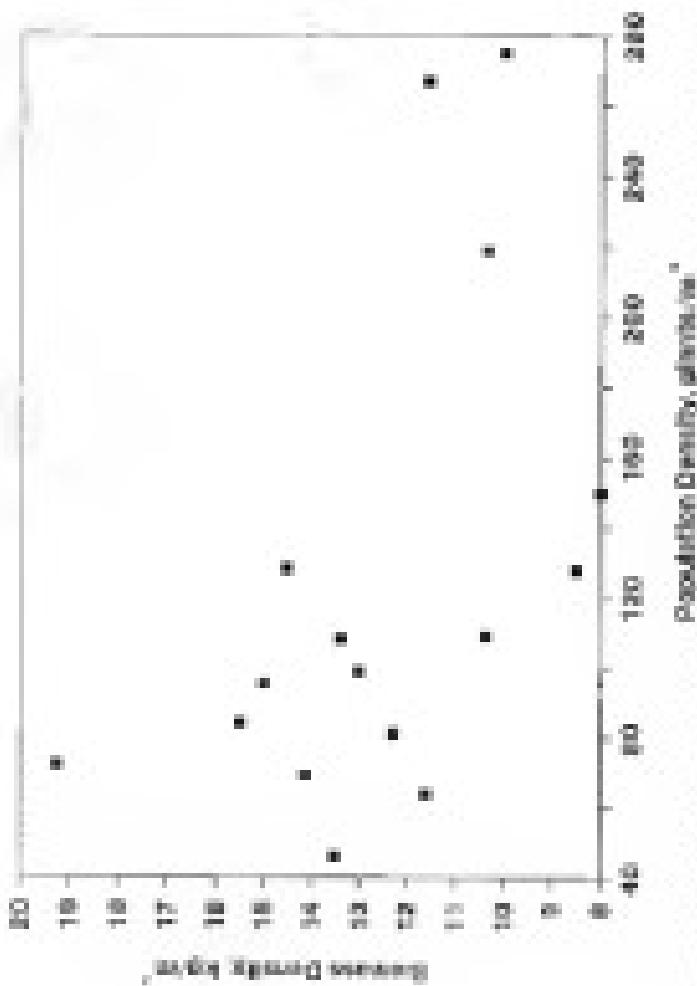


FIGURE 3.7 The relationship between relative density of water and relative density of various liquids.

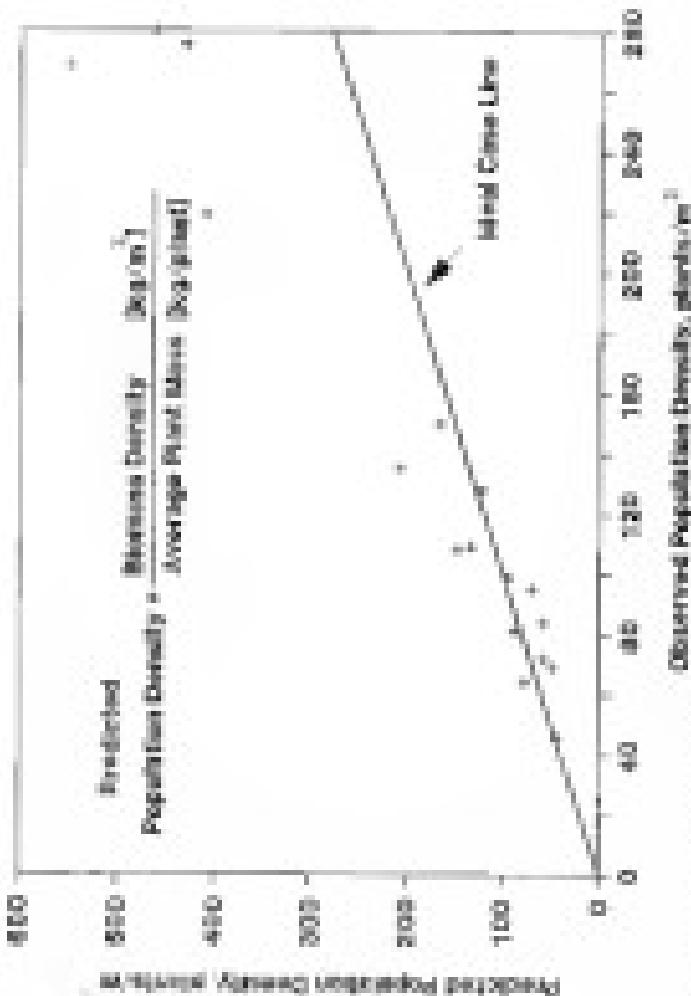
population densities less than 100 plants/m², but it is very unreliable for aggregations of small plants or large populations (Figure 3.8). Small plants usually indicate a population that is not well established, so that more reliable values for plant size can be difficult to obtain (Figure 3.9). A more accurate prediction of plant number was established (Figure 3.10). The relation was developed from Turner's (1986) observations that biomass directly increased with particle length. The best square relation is a nonlinear relation involving the logarithm of average plant height. Biomass density and average plant length are the independent variables. Both of these relations were established for populations with short roots; hence, they should not be used for long-rooted plant species.

Sensitivity and Accuracy

Net frequency in this study varied from 20 to 100 % and corresponding from 90 to 100 kg (Table 3.1). Hartland and Hiltner (2000), on the other hand, found that seedling frequency varied from 17% to 40% kg. The larger value, 100, probably represented data from an unusually compact vegetation.

Net frequency did not correlate well with standing crop or population density, although frequency appeared to increase somewhat with stand area density. The relations between net frequency and stand area density became evident when the data were parameterized by the index of sensitivity to net frequency (Figure 3.12). The linear regression relations are presented in Table 3.2. The highest frequency of any plant area was density observed when sensitivity values were less than 0.4 and the lowest net frequency occurred when the sensitivity

Figure 1. Population Density - Primary Residential Density (per square kilometer) and Secondary Residential Density (per square kilometer) in the United States by State, 2000.



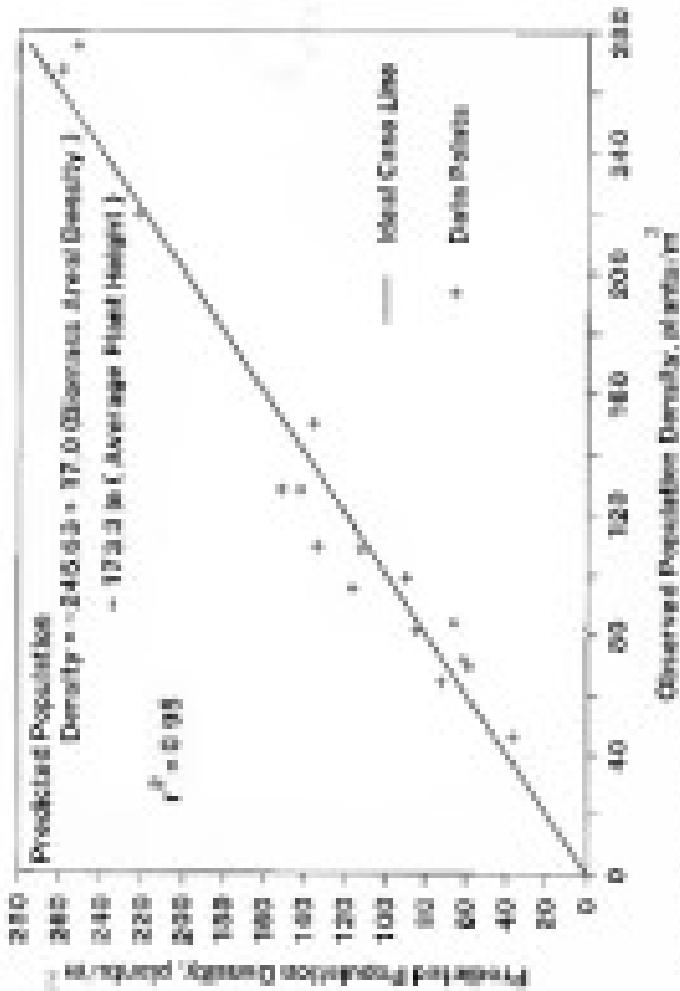


Figure 1.8

This figure illustrates trends for predicted versus observed population density. Population density depends on small towns' and urban centers. This scatter line represents the theoretical linear relationship between predicted and actual measurements.

Figure 5-10. The percentage reduction in each tree species' basal area and volume of trees remaining after different rates of thinning.

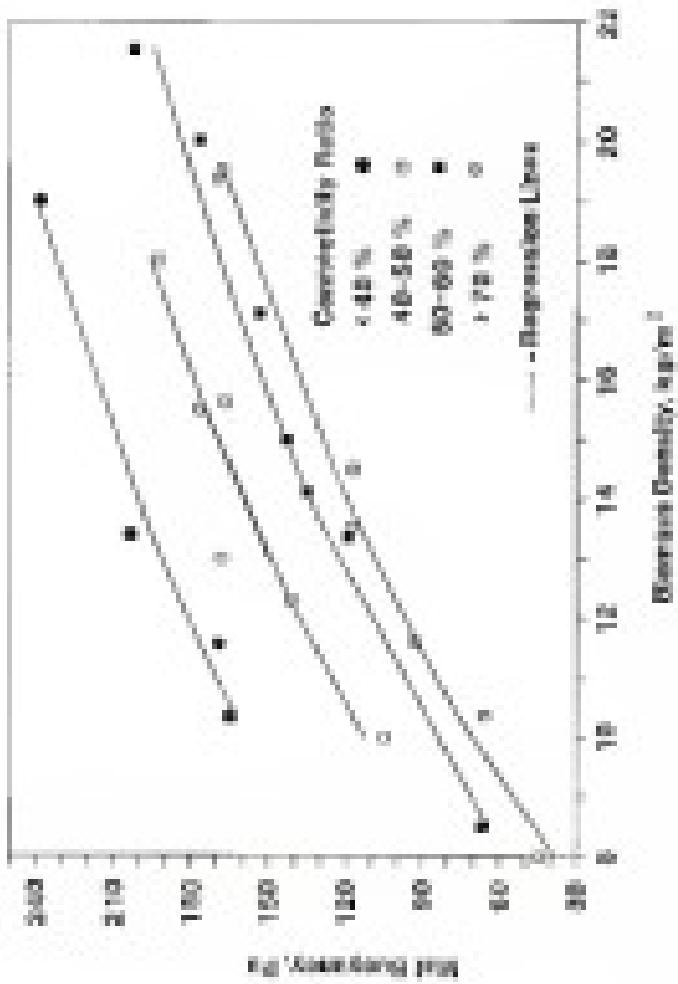


Table 3-1. Values of net capacity, availability, and utilization ratio

Net Capacity	Connectivity		Actual Capacity	Average Line Length
	Po	Pn	L	avg/L
100 0	20 0	10 0	10 0	10 0
100 1	40 1	20 0	11 9	11 0
100 2	60 0	30 0	12 4	12 0
100 3	80 0	40 0	13 0	12 0
100 4	90 0	50 0	13 0	12 1
100 5	100 0	60 0	13 0	12 2
100 6	110 1	60 0	13 3	12 0
100 7	120 0	60 0	13 0	12 0
100 8	130 0	60 0	13 0	12 0
100 9	140 0	60 0	13 0	12 0
100 10	150 0	60 0	13 0	12 0
100 11	160 0	60 0	13 0	12 0
100 12	170 0	60 0	13 0	12 0
100 13	180 0	60 0	13 0	12 0
100 14	190 0	60 0	13 0	12 0
100 15	200 0	60 0	13 0	12 0
100 16	210 0	60 0	13 0	12 0
100 17	220 0	60 0	13 0	12 0
100 18	230 0	60 0	13 0	12 0
100 19	240 0	60 0	13 0	12 0
100 20	250 0	60 0	13 0	12 0
100 21	260 0	60 0	13 0	12 0
100 22	270 0	60 0	13 0	12 0
100 23	280 0	60 0	13 0	12 0
100 24	290 0	60 0	13 0	12 0
100 25	300 0	60 0	13 0	12 0
100 26	310 0	60 0	13 0	12 0
100 27	320 0	60 0	13 0	12 0
100 28	330 0	60 0	13 0	12 0
100 29	340 0	60 0	13 0	12 0
100 30	350 0	60 0	13 0	12 0
100 31	360 0	60 0	13 0	12 0
100 32	370 0	60 0	13 0	12 0
100 33	380 0	60 0	13 0	12 0
100 34	390 0	60 0	13 0	12 0
100 35	400 0	60 0	13 0	12 0
100 36	410 0	60 0	13 0	12 0
100 37	420 0	60 0	13 0	12 0
100 38	430 0	60 0	13 0	12 0
100 39	440 0	60 0	13 0	12 0
100 40	450 0	60 0	13 0	12 0
100 41	460 0	60 0	13 0	12 0
100 42	470 0	60 0	13 0	12 0
100 43	480 0	60 0	13 0	12 0
100 44	490 0	60 0	13 0	12 0
100 45	500 0	60 0	13 0	12 0
100 46	510 0	60 0	13 0	12 0
100 47	520 0	60 0	13 0	12 0
100 48	530 0	60 0	13 0	12 0
100 49	540 0	60 0	13 0	12 0
100 50	550 0	60 0	13 0	12 0
100 51	560 0	60 0	13 0	12 0
100 52	570 0	60 0	13 0	12 0
100 53	580 0	60 0	13 0	12 0
100 54	590 0	60 0	13 0	12 0
100 55	600 0	60 0	13 0	12 0
100 56	610 0	60 0	13 0	12 0
100 57	620 0	60 0	13 0	12 0
100 58	630 0	60 0	13 0	12 0
100 59	640 0	60 0	13 0	12 0
100 60	650 0	60 0	13 0	12 0
100 61	660 0	60 0	13 0	12 0
100 62	670 0	60 0	13 0	12 0
100 63	680 0	60 0	13 0	12 0
100 64	690 0	60 0	13 0	12 0
100 65	700 0	60 0	13 0	12 0
100 66	710 0	60 0	13 0	12 0
100 67	720 0	60 0	13 0	12 0
100 68	730 0	60 0	13 0	12 0
100 69	740 0	60 0	13 0	12 0
100 70	750 0	60 0	13 0	12 0
100 71	760 0	60 0	13 0	12 0
100 72	770 0	60 0	13 0	12 0
100 73	780 0	60 0	13 0	12 0
100 74	790 0	60 0	13 0	12 0
100 75	800 0	60 0	13 0	12 0
100 76	810 0	60 0	13 0	12 0
100 77	820 0	60 0	13 0	12 0
100 78	830 0	60 0	13 0	12 0
100 79	840 0	60 0	13 0	12 0
100 80	850 0	60 0	13 0	12 0
100 81	860 0	60 0	13 0	12 0
100 82	870 0	60 0	13 0	12 0
100 83	880 0	60 0	13 0	12 0
100 84	890 0	60 0	13 0	12 0
100 85	900 0	60 0	13 0	12 0
100 86	910 0	60 0	13 0	12 0
100 87	920 0	60 0	13 0	12 0
100 88	930 0	60 0	13 0	12 0
100 89	940 0	60 0	13 0	12 0
100 90	950 0	60 0	13 0	12 0
100 91	960 0	60 0	13 0	12 0
100 92	970 0	60 0	13 0	12 0
100 93	980 0	60 0	13 0	12 0
100 94	990 0	60 0	13 0	12 0
100 95	1000 0	60 0	13 0	12 0
100 96	1010 0	60 0	13 0	12 0
100 97	1020 0	60 0	13 0	12 0
100 98	1030 0	60 0	13 0	12 0
100 99	1040 0	60 0	13 0	12 0
100 100	1050 0	60 0	13 0	12 0

NOTE: The last two entries represent data from long-hauled places.

faster than greater than 0.7. But buoyancy is affected by connectivity ratio because the ratio reflects the types and conditions of phases comprising a soil. High connectivity ratio indicates the phases are supported by particle entanglement or are interconnected by adhesion, while a small ratio indicates many particles are isolated and individual phases are supporting themselves. Particles with plastic and phases of low connectivity app cause slow capture to periodically intersect, because open conditions lead to their formation to phases of low value [See 100]. Low connectivity causes also plants are separated. Plastic behavior and buoyancy, because the specific gravity of a plant is 0.226 or according the density of a regular particle.

Table 3.2: Relationship between soil buoyancy [90] and resulting biomass density [90]

Equation	Connectivity ratio	ρ
$B = -0.01 \phi + 104.5 \text{ kg/m}^3$	0-10	0.00
$B = -0.01 \phi + 136.5 \text{ kg/m}^3$	10-20	0.00
$B = -0.01 \phi + 145.5 \text{ kg/m}^3$	20-30	0.00
$B = -0.01 \phi + 227.4 \text{ kg/m}^3$	> 40	0.00

The values for buoyancy were calculated for aggregated phases and they were smaller than those reported for sharp-edged phases with similar connectivity and resulting densities. This is expected because

long-stemmed plants have several times more biomass underneath the roots than compared to shorter-rooted systems.

Rooting.

Near-groundwater content, porosity of soil, and although complex aerenchyma, they can be defined by leaves, rhizomes, roots and stems. Free-living bacteria help support the plant under stress conditions by exchange with leaves from other roots. Root length is correlated to plant mass, and root length to rootless root length.

In inhibited populations of water hyacinth have normally distributed plant mass, while suppression of small plants or plants subjected to regular harvests are typical of many types of plant groups.

Population density is correlated with plant length and floating biomass density.

Groundwater and soil buoyancy are independent properties that reflect the mechanics of the vertical stability of roots and plants.

CHAPTER IV SOLID AND FLUID MECHANICS

It occurs to a collection of interacting elements for which there are measurable relationships among the variables. A water hyacinth plant can be subjected to friction and elastic forces resulting from interaction of plants with each other, surface, floating leaves of soil or earth bottom, and drag from the interaction of plants with water. Therefore, problems concern elements are related to body forces (gravitational and gravitational) and surface forces (friction and shear). To express the shear and friction forces in form of system elements, constitutive equations are required. They depend only on the characteristics of the physical material that compose an element and on its geometry, and they show the basic relationships between the variables (e.g., Bingham law in solids and Newton viscosity law in fluids).

Hydrodynamic Parameters.

The purpose of this section is to develop the mathematical relationships between drag and towing velocity as a function of size of an aggregation of water hyacinths and plant size and shape.

Reduced Literature.

There, thickness and form qualities of most bodies that are subjected to water. When they are placed in flow, they are subjected to drag. Drag is composed of two components: friction drag resulting from tangential stresses caused by friction of the fluid against the interface, and pressure or form drag, resulting from pressure forces exerted by the fluid normal to the surface of the body. The latter is predominant at high Reynolds numbers. Drag on a body can be determined empirically for simple shapes and flow not especially for slightly more complicated geometries, but it is usually determined experimentally for bodies in turbulent flow. Consequently, the formula for overall drag is written in the following form:

$$D_f = \frac{C_d A \rho V^2}{2} \quad (1.1)$$

where D_f is drag force, A is the area of the body projected in the direction of flow, ρ is density of the fluid, C_d is a drag coefficient and V is the velocity of the fluid relative to the body which may move or be stationary.

A similar drag formula was initially proposed by Reuss, but the original proportionality factor (instead of C_d) did not always coincide with experimental data. This problem was resolved when a function of Reynolds number was used to replace the original proportionality factor (Prandtl and Tietjens, 1934). Reynolds number based calculation of drag in a Newtonian fluid on similar bodies which have the same orientation with respect to the free stream velocity. The relation between C_d and Reynolds number must be

determined for turbulent flow only experimentally. Furthermore experiments prove that along the wall drag on a body consists exclusively of pressure drag (i.e. for high Reynolds numbers), the drag coefficient is large and independent of Reynolds number. In the case of friction drag, the drag coefficient is a function of Reynolds number, and it is usually small in value.

Because are the principal projections into water, they are optimally shaped. Due to the shape of riblets-water boundary it may not be isolated from the analysis for a series of riblets (e.g. in a flow channel). The information concerning the fluid properties of major boundary can be measured from a series of profiles of the boundary by specific probes, which are assumed to represent roughness elements. These topics will be reviewed next.

Reynolds number of riblets. Since the shape of riblets can be assumed to be optimised, the following information is available over a bank of riblets in particular to the description of water boundary behavior. Schlier [1977] suggested shape criteria, η_0 , to meet the case:

$$\eta_0 = \frac{P_R D}{\delta} \quad (6.2)$$

where P_R stands now that ρ is the mass of major contributions introduced by flow through a bank, and δ is a relative factor which is a function of Reynolds number and the ratio of transverse to longitudinal spacings.

We found that friction coefficients for different arrangements of riblets were functions of Reynolds number (based on optimised

diameter), the number of major cross-sections measured in flow through the tank, and the ratio of distances to longitudinal spacing. The number of major cross-sections depended on the configuration of the cylinders: for bundle arrangements, p meant the number of rows for staggered arrangements, with sections taken in transverse staggered systems; p equaled the number of rows and the staggered arrangement with minimum flow area (i.e., diagonal transposition) spacing of cylinders between cylinders; p equaled the number of rows times one. Generally, the friction factor was measured until Reynolds number exceeded 20,000. Above 10,000, the general characteristic curve for friction factor resulted decreased.

Later, the equation for total head loss due to friction was refined to consider the friction factor f_{D} in Darcy's form (Equation 4-4) (Dixey and Ladd, 1964 and Rehme and Parker, 1982).

Total head loss (Dixey and Ladd, 1964) =

$$\Delta h_f = f \frac{L}{d_p} \frac{V^2}{2g} \quad (4-4)$$

where f is Darcy friction factor, V is flow velocity, $\frac{\Delta h_f}{d_p} = \frac{4 f L}{3}$, L is total distance, d_p is total flow area summed over, and g is the gravity. Both A and L are measured from the leading edge of the first cylinder row to the trailing edge of the last row of cylinders, and V equals the hydraulic radius (Hydraulic Radius = $\text{Flow area} / \text{Perimeter of cylinder}$).

boundary elements and boundary resistance. Boundary resistance is often presented in an alternative form that was derived by dimensional analysis [Lauk, 1960]:

$$\frac{R_f}{\rho} = \frac{L}{g} + \left[\frac{\rho L}{\eta} \right]^{1/2} \cdot R^2, \quad (4.1)$$

here L is the characteristic length, η is intrinsic viscosity, ρ is average density and g is the acceleration due to gravity; dimensionless Reynolds number is defined by $\frac{\rho L h}{\eta}$. For soil, the formula that is often used to determine the size of turbulent flow through pipes is identical to the one in Equation 4.1. Lauk [1960] suggested that the value of R appears to depend on the smoothness of a surface.

Stochastic simulation analysis of flow retardance by rough soil plates have shown that plates in almost all cases are roughness elements. Basic ranges of their profiles for roughness elements have been established as the ratio of height and spacing of roughness elements [Sel'sadikov, 1979]. There are 1) the optimally smooth regime where the size of the roughness is so small that all perturbations are contained within the viscous sub-layer; 2) the transition regime where perturbations influence the laminar sub-layer and the transition is compared with a smooth pipe. In the study in flow drag experienced by the perturbations in the turbulent boundary layer and finally 3) the completely rough regime where all perturbations exceed outside the viscous sub-layer and the resistance is due mainly to flow drag. These regimes are determined by the following criteria:

a. Hydraulically smooth

$$\frac{k_s V_{\text{D}}}{R} \leq 0.01 \quad R = \text{Hydraulic radius}$$

b. Transition

$$0.01 \leq \frac{k_s V_{\text{D}}}{R} \leq 0.05 \quad R = \text{Hydraulic radius}$$

c. Coarsely rough

$$\frac{k_s V_{\text{D}}}{R} \geq 0.05 \quad R = \frac{b}{1 - \frac{h}{b}}$$

where k_s denotes the grain size in diameter s and roughness, V_{D} is the friction velocity defined by $\sqrt{gk_s}$, R is the hydraulic radius, $\frac{k_s V_{\text{D}}}{R}$ is often referred to as the wall Reynolds number, and V_{D} is shear stress at the wall. Transition factor (Hewitt-Holman) is related to shear stress by $\sqrt{gk_s} = \sqrt{gk} V$. Furthermore, k_s has been related experimentally to Manning's n (Bathurst, 1990)

$$n = (1/0.02) k_s^{1/0.02} \quad (4.4)$$

with value $n^{1/0.02}$ and $k_s^{1/0.02}$ in roughness in meters such that 95% of the material is of lesser size. The equation is valid for $0 < \frac{b}{h} < 1000$. Manning's n is an empirical coefficient, its value increases with increasing roughness.

In reference to the over vegetated plants, Rao and Paluri (1990) demonstrated by experiments that the degree to which various channel roughening agents and flow depend largely upon the degree to which vegetation is low and flattened by the flow, which in turn depends mainly upon physical characteristics of the vegetation: the nature of growth, and the velocity and depth of flow. The plants were different kinds of grasses and herbs. They specifically show that flow was sufficiently slow and has to bend over and entangle the channel

regression, a greater velocity resulted due to a greater slope of the energy grade line resulted in lower resistance to flow. Manning's n values for short and medium-height vegetation appeared to decrease to a nearly constant value of about 0.38; as depth of flow was increased or over the plants. Values of n for different soil plant species varied widely for all depths of flows. In regard to population density for a specified flow depth, a data series of vegetation (300 plants/m²) offered twice as much resistance to flow as a bare stream bank (17 plants/m²). The authors developed a graphical method for the determination of shear stress versus nR . To do so, they plotted Manning's values versus nR (velocity times hydraulic radius) because they perceived resistance to be a function of the degree of flattening of the vegetation, which is influenced by velocity and depth of flow. As a result, they found that until a given regressive channel lining became null (zero), the value of nR indicated the degree of resistance the living offered. However, when the vegetation was sparse, or nearly so, and well submerged, it became potentially unnecessary and caused no correlation with nR .

On the other hand, Beven and Favis (1973) determined the values for the local friction factor, κ_0 for sand and the other for grass heights. They found the friction for the sand and young English grass is a function of a relative roughness factor, while for grass plant roughness, friction factor was a function of nR . The experimental results were in the form of graphs. This apparent discrepancy in the two interpretations could relate to the manner the vegetation was submerged or to the depth of flow. Although, it appears that Brown and Long were correct in their conclusion because 1) their theory of

In fully rough regime in a channel predicts the drag coefficient to be independent of Reynolds number, or drag varies with R^2 and is independent of viscosity, and 2D open vegetation is more likely to cause a fully rough flow regime because the roughness elements are easier to attain larger values for the total Reynolds number.

Many investigators have found that resistance is an effective coefficient increasing with the extent of aquatic biomass present (Petryk, 1980; Rouse et al., 1963; Rouse, 1998; Petryk and Beaupre III, 2002; Saito and Saito, 1992 and Beaupre et al., 1994). Petryk and Beaupre III (2002) defined density of aquatic vegetation by the respective area per unit length of channel per unit area of flow as expressed by the following equation:

$$\text{Density of vegetation} = \frac{A_1}{L^2}$$

where A_1 = projected area unitized area of vegetation to the flow direction in a channel of length L and cross area A .

The effect of plant density on flow resistance can be related to roughness spacing. Rouse (1963) studied longitudinal spacing of strips in a channel. He found that the ratio of roughness spacing to roughness height was an important measure of resistance, (i) relative roughness, and (ii) was greatest regardless of the value of the roughness width or height ratio. Saito and Saito (1992) studied both longitudinal and transverse resistance spacing, and he used dimensional analysis to develop the following spacing parameter:

$$\text{Spacing parameter} = \frac{A_1}{(a + b)} \quad (4.6)$$

where a = longitudinal spacing of roughness surface, b = roughness

height, b = roughness width and λ = constant specific length. The flow was considered to be in the completely rough regime. Data were analyzed in terms of the Kutter-Peacor's expression of turbulent flow near a rough boundary, and the following equation was fitted to the data:

$$\frac{dy}{dx} = 0.01 \log \frac{y_0}{x_0} + C_0$$

where C_0 is the drag discharge coefficient = $10^{10} \alpha$, x_0 is normal depth, C_0 is experimental constant dependent on roughness spacing, b is height of roughness element, and y is acceleration of gravity. Similarly, using the equation for longitudinal velocity profile near a turbulent wall, Rouse et al. (1969) obtained the equation for flow velocity in a vegetated channel:

$$\frac{dy}{dx} = C_1 + C_2 \ln \frac{x}{x_0}$$

where C_1 and C_2 vary with the vegetative density and the flexibility of the vegetation, x is the gross cross sectional area of the channel and x_0 is the area of vegetative area section.

In summary, the most relevant concepts discussed in this section for application to river hydraulic are:

- a. Theory and experiments have shown that bigger plants and denser vegetation produce a larger drag coefficient;
- b. Empirical relations for percent dry surface losses of cylinders in cross flow have shown that cylinder diameter and size of

the bank aspect ratio, cylinder length, bank length and bank width influence friction factor.

- a. For Reynolds number (based on tube diameter) less than 10,000 the friction factor of a bank of cylinders to cross flow is independent of Reynolds number.
- b. Transverse and longitudinal spacing of cylinders to cross flow and of roughness projections in flow affect drug coefficient.
- c. Please note that some changes in flow after four passes are to the first cross plane.
- d. Experiments have proven that when regrinding is used, the drug coefficient is independent of Reynolds number, and it is dependent on a relative roughness parameter.

Non-ideal flow characteristics.

In this section, we approach the first valid for a rectangular arrangement of plates of any population density at spacing and orientation given in flow to develop in this section. Neither theoretical nor analytical solutions appeared feasible for the problem of inter-tubular flow steady flow due to the assumption of 1) circular and rectilinear curvatures and 2) the non uniform shear stress. Therefore, an analytical solution based on rigid, rough flat plates is not appropriate. Consequently a drug model was developed based on Darcy's drug equation (Equation 4.0), and drug coefficient were obtained experimentally.

To begin, a model for drug must be dimensionally correct. The drug equation (Equation 4.0) was used after it was modified by the incorporation of the value 0 into the drug equation:

$$D_f = \rho \cdot g \cdot C_d \cdot V^2 \quad (6.6)$$

where D_f equals the drag force (Kilogram), ρ equals the density of water approximately equal to 1000 kg/m³, A equals the projected area in Plan (m²), C_d equals a dimensionless drag coefficient, and V is the testing velocity (m/s). Although Equation 6.6 implies C_d is a function of Reynolds number, evidence in the literature indicates that the drag coefficient for an aircraft, right proportionality of water speed, is independent of Reynolds number. If applied, C_d of water speed to is independent of Reynolds number the testing velocity would enter the model only as V^2 . In substantiation of this, the Reynolds number was increased (diameter of cylinder range from 0.24 to 0.48 m). The Reynolds number for water speed test based on a 0.24 m diameter and a testing velocity of 0.4 m/s is calculated below:

$$\text{Reynolds number} = \frac{\text{Diameter of Cylinder} \times \text{Testing Velocity}}{\text{Kinematic Viscosity}}$$

$$= \frac{0.24 \times 0.4}{0.001008 \times 10^{-6}} = 975.4$$

However, for 0.48 m diameter, the Reynolds number is approximately 18,880 at 0.4 m/s, which are usually the observed upper limit of stable testing conditions. Testing and result changes can be expected to some Reynolds numbers below the lower limit in which drag of cylinders in free flow is dependent on Reynolds number.

To use the drag equation, the friction factor must be defined. The used friction value was assumed to approximately equal the projected

area of a rhomb (A_p) subtended by the total vector of plastic (Δ_p) is an expression as

$$A_p = R_p \cdot D_p \cdot l_p = R_p \cdot k_p \quad (4.7)$$

R_p equals the average diameter of the rhombes and k_p , average length.

The shape of a unit rhombus will also influence the drag coefficient since drag coefficients are only valid for geometrically similar bodies of the same orientation in relation to the flow direction velocity. Therefore, it must suffice to state as how many plastic along the flow direction will have a different drag coefficient than a rhombus one. The shape factor covers the geometrical variation mostly on μ (Equation 4.4) and continuously on ΔA_p (Equation 4.6). For a rectangular one of same length, the simplest geometrical shape factor is the ratio of the two dimensions, i.e. width W to length L .

For this study the transverse and longitudinal spacings of every specific width W are not one constant number. As a result, the drag coefficient was considered to be a function of a shape factor and non-dimensional Reynolds number. Thus, the proposed drag model for unit rhombuses becomes:

$$D_p = \mu R_p D_p l_p C_D \frac{V^2}{2} \quad (4.8)$$

where,

$$C_D = 4 \left[\frac{\frac{W}{L} - T}{2} + \frac{T}{L} \right]$$

This proposed equation is valid for any rectangular aggregation of water hyacinth without the need for graphs or tables to determine C_0 ; provided, of course, that C_0 is constant superficially.

Materials and Methods.

This section presents the procedure that was used to 1) test the hypothesis that the drag coefficient of a water hyacinth is independent of Reynolds number and 2) determine the empirical relation of C_0 to wet porosity.

Materials.

Aggregations of water hyacinth composed primarily of small, medium, or large-sized plants were used. Small plants averaged 20 cm in length; medium plants averaged 60 cm; and large plants averaged 100 cm. However, the water was deep enough, a buoyancy test was performed on the site before it was used as the testing area. Water hyacinths were obtained from the University of Florida, River Research Unit, River's Arm, and Lake Alafia. The tanks are located near the University of Florida. After completion of each experiment, never plants were measured for wet plant length and diameter, buoyant particle length, plant length and root length. Root length varied according to location and ranged from 3 to 21 cm. The presence of roots and rhizomes was noted.

Aggregations of plants from the River Unit were retained in the testing area to determine: First, if 0.1 by 0.1 m area of plants was retained, then, a frame of wood and frame wire was positioned underneath the plants. Lifting the frame raised the plants from the water in their normal position. Because the water was too deep on

On the dates, the test results were obtained to show and had just been
in the Press:

Brake experiments.

To measure the drag coefficients of water specimens, two experiments were performed on different sizes of rectangular imitations of water specimens. The tests were performed in the hydrostatic tank, Civil Engineering Department, University of Florida. The testing equipment was built as adjustable three bearing floating frame-like structure, a powered trailer by water velocity sensors and instrumentation (Figure 4-6).

The floating structure consisted of one frame, an outer supporting structure of 30 x 30 x 1.5 m square, steel tubing on the 100 mm PVC perimeter and an inner rectangular frame of 25 x 25 x 1.5 m square, steel tubing. The inner frame weighed approximately 55 kg and was suspended from the outer frame by four rectangular overhanging beams interconnected with counter-clockwise-hinged, loaded strain gauge bridges. The clearance of the outer frame above the perimeter was incrementally adjusted so that the inner frame just cleared the water. Arrangement of the two frames assured that water specimen retained in the inner frame would not touch the upper frame. The outer frame was fitted with 8 m rod piles projecting 100 mm below and 100 mm above the frame to prevent planar free floating and/or rolling under the frame and, 10 m arranged to be sufficiently long (4.0 m long and 1.0 m wide, 1.0 x 0.12 m, 1.2 m x 1.2 m, 1.2m x 0.6 m, and 0.6 m x 0.6 m cross-sectional areas). The 0.6 m x 0.6 m sections were laid down placed all around

Figure 4.8 Testing approach (modified from <http://www.isti.cnr.it/it/2009/07/01/test-strategies-and-test-plans>)



it because the plants emerged from the older during germination experiments.

The length of each cuttling root was 0.30 m and the width and thickness were 0.08 and 0.05 cm respectively. The two cuttling roots were attached to each side of a cuttling basal base. Two rigs bridge strengths, one for each end of the frame, were used to measure the forces against the frame. The force measuring system was calibrated by using a spring scale and by recording the corresponding readings (Figure 4.1).

Rooted velocity was recorded by the velocity Profiler app. Velocity sensors (A: the Beaufort current meter) attached to the seedling, were used to the basal roots. After the sensors failed to function properly, distances along the cutting path were marked off and then as travel a distance was recorded through the aid of video equipment.

The force and velocity signals from the two rigs were recorded by a Campbell Scientific 1207 data logger with observations recorded every second.

Before some of the tests, a grid of thin sticks were placed over the soil at places for subsequent observations of plant movement in relation to each other. The tests were videotaped. Some underwater observations were visually recorded through a glass window in the side of the channel.

A typical experiment proceeded as follows. Plants were placed like the installation frame. The frame was pulled by a trawler through a 0.4 m wide channel with the water depth kept at 0.10 m. Plants were

Figure 4.1 In this group were selected 10 individuals whose wings were suspended from the upper tree and recorded on the lower tree. The lower tree maintained silence. Singing activity was measured by a current meter.



reached a constant speed for a distance of approximately 11.0. Testing speed was increased from 0.1 to 0.6 m/s until the plums started to roll under the centrifugal作用. Once this happened, the test was ended and the sample was subdivided. Some of the plums were reserved to determine their total mass. Several plums were divided and weighed to determine the quantity of biomass in the sample. Testing rates were repeated on sequentially smaller disks until no more were tested.

Force and velocity data for each crop were regressed according to the equation:

$$F_p = C_p V^2 \quad (4.10)$$

where F_p equals total drag force in Newtons, C_p is a measure of penetrability equal to the product of ($\pi R_p L_p L_y C_{\text{fr}}$) from Equation 4.8 and V equals testing velocity. Drag force was determined to be the force on the centrifuge bags after the plums had stopped accelerating and the forces due to the acceleration stage of the test had distributed to a slight fluctuation. The parts of the testing trials were labelled the steady and steady stages (Figures 4.1 and 4.2).

The data were analysed for upper sample by experimental error can be quantified when the distribution approximation is normal distribution (Berkely, 1994). The principal source of experimental error was the data logger. Built in the test panel of the data logger was a 0.003-m, pitch resolution limit on axes of ±0.001. However, all the forces due during the steady state conditions of a testing trial

Figure 4.2. A scatter plot showing the relationship between the total pressure of the plasma and the total density of the plasma. The data points are shown as black dots, and the trend line is shown as a solid line.

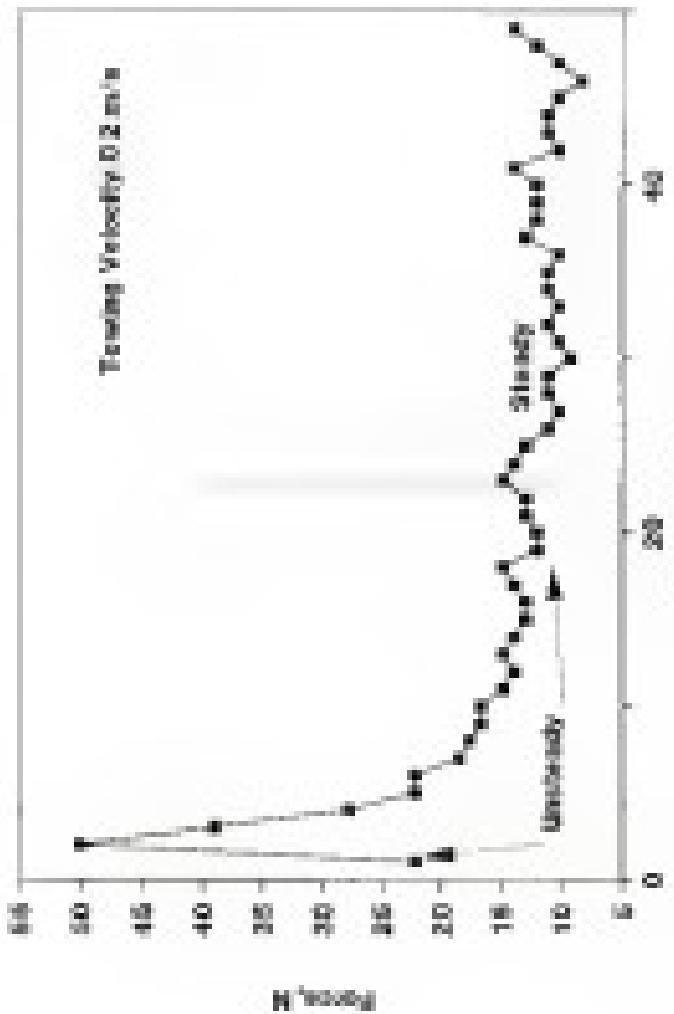
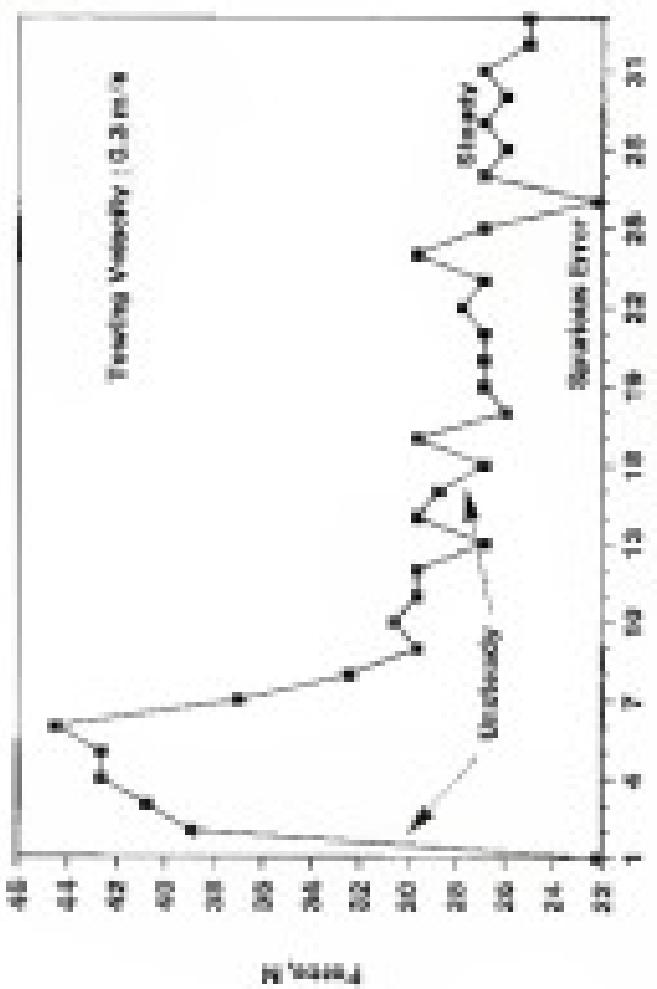


Figure 1-3. A sample graph of the mean and individual variability measures of the 12th-grade students' performance in the second semester.



was employed for the two testing velocities (0.2 m/s and 0.3 m/s) and the values were 0.262 ± 0.025 & 0.261 ± 0.025. If it is assumed that the distribution is normal, consequently, the force distributions can be considered to be normal. A histogram of the drug force distributions is shown in Figure 6.4. The dispersion of drug force about the mean average was measured by standard deviation when the distribution was normal. Standard deviation of drug force, S_D , was calculated using data from the ten tests while standard errors of S_D (Equation 6.10), drug force estimated from the regression model, and of the coefficients, C_i (Equation 6.11) were computed for every test. Since the International Standard Organization has recommended that the probability level used should be at the 90 percent level (ISO, 1978), error was reported using the corresponding t statistic multiplying either the standard error or deviation.

Standard Error of Estimate, S_E (Dempsey, 1978).

$$S_E = \left[\frac{\sum (x_i - \bar{x})^2}{n-1} \right]^{\frac{1}{2}} \quad (6.10)$$

where x_i is the deviation of the tested value from the computed value taken from the curve of reference at \bar{x} ; $\bar{x} = \bar{x}_D$ and n the sample size.

Standard Error of the Coefficients (Dempsey, 1978) is,

$$S_{C_i} = \frac{(n-1)^{\frac{1}{2}}}{\sqrt{(n-1)x^2 - (1/t^2)}} \quad (6.11)$$

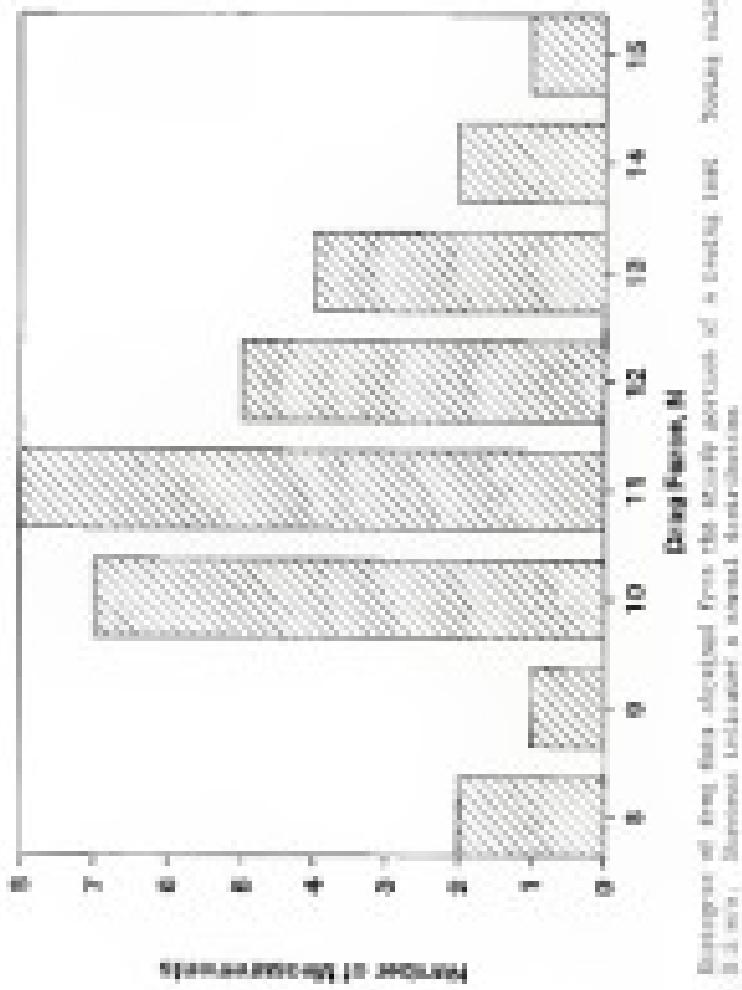


Figure 1. Percentage of the total explained variance of surfaces of a Gmelin louse (Trombicula gmelini) using Principal Components (PCP), Partial Least Squares (PCL) and Partial Least Squares with Principal Components (PCLP).

Spatial errors were discarded (Figure 4.3) after data lay more than ± 1 standard deviation from the sample mean. The value of n depends upon the sample population size [Preston, 1980]. For example, for a population of 100, n equals 10.

Systematic error in the three types could be related to the design of the test frame. Thus smaller planes within the test frame. The hypothesis is that this was considered small, because the diameter and number of planes were small relative to those of the choppers. However, the drag of these was still less than 1.00 m drag by 0.0 m wide test frame, which had open planes, was measured as 1.00 m at a free-stream velocity of 0.0 m/s and 1.00 m at 0.0 m/s. The drag on the smaller frame was nearly 30 % higher than the drag on other frames. Thus an error in any testing system, as their variance from the experiments was not tested separately. Also, these measurements could be susceptible to systematic error if the calibration of the wind tunnel was inaccurate. The planes were distributed below using different input voltages, varying neither end stages of drive, and the calibration factors remained the same. Finally, another cause of systematic error is the number of significant digits the data logger was to store force readings. Too few and very small forces would not be recorded accurately by the data logger.

Results and discussion.

At the beginning of a testing run, planes tended to hang together, shortening the length of the run in days. Despite of the placement of grid lines on top of the hydraulic damper and lack of competition. That is that before the grid lines at the leading edge

of the not treated stayaway class at the rear end. Once the dried not treated samples spent the grid lines were equally spaced again but closer than in comparison of the not.

Comparative increased with reading velocity until "full water". "full water" measured at a velocity exceeding 0.33 m/s. At this point, the plates on the leading edge started to roll forward under the water. Once water over the sample started to flow over underneath the water plates lifting up the surface. In this way, the plates on the surface passed over elongated sample and, in turn, rolled under when they were positioned at the leading edge (Figure 4.8 → 4.12). In "full water", the force on the not leaves greatly increased, and the portion of the force = reduced area per unit of "stability" was absent (Figures 4.8 and 4.12).

Some young birds were reported to make attempts to dislodge the seeds. Once this reported seen full on the water bird (Figure 4.10), however, the seeds could be dislodged under similar conditions.

These plates are extremely...that if this being dissolved in the water, yes, they are very flexible. Unfortunate observations showed the result to be completely great at a testing velocity of 0.27 m/s and moving back and forth at lower speeds. But the ratio were prime, the response of drag was due to resistance.

Results of experimental results

A model (11.0 experimental) does provide the expected deviation of the theoretical resistance value to the range of the expected standard error of the predicted value. To determine the efficiency of the proposed model, these fitting values of the drag forces were plotted against the square of the testing velocity for the sample series. The

Figure 8.3 When again were found, they supported. Amount of compression decreased with reaction velocity. At 2.1 m/s, compression was slight.



Figure 5.4 The red star located at $\delta\alpha = 0^\circ$ and $\delta\beta = +28^\circ$ is composed approximately 20 stars.



Figure 4.1 The four main types of software and their associated characteristics



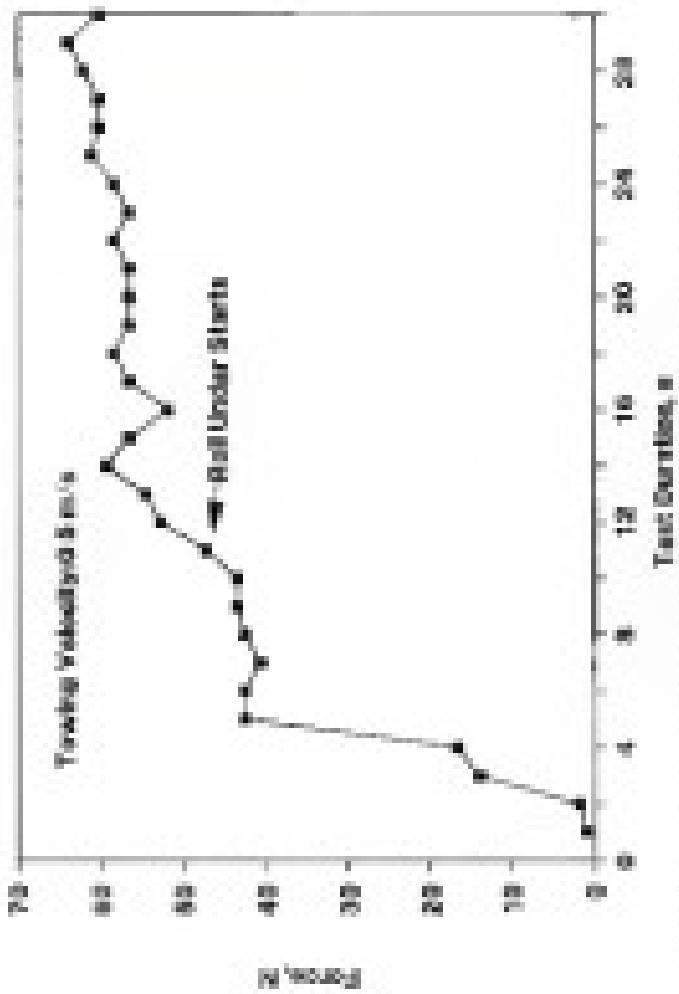
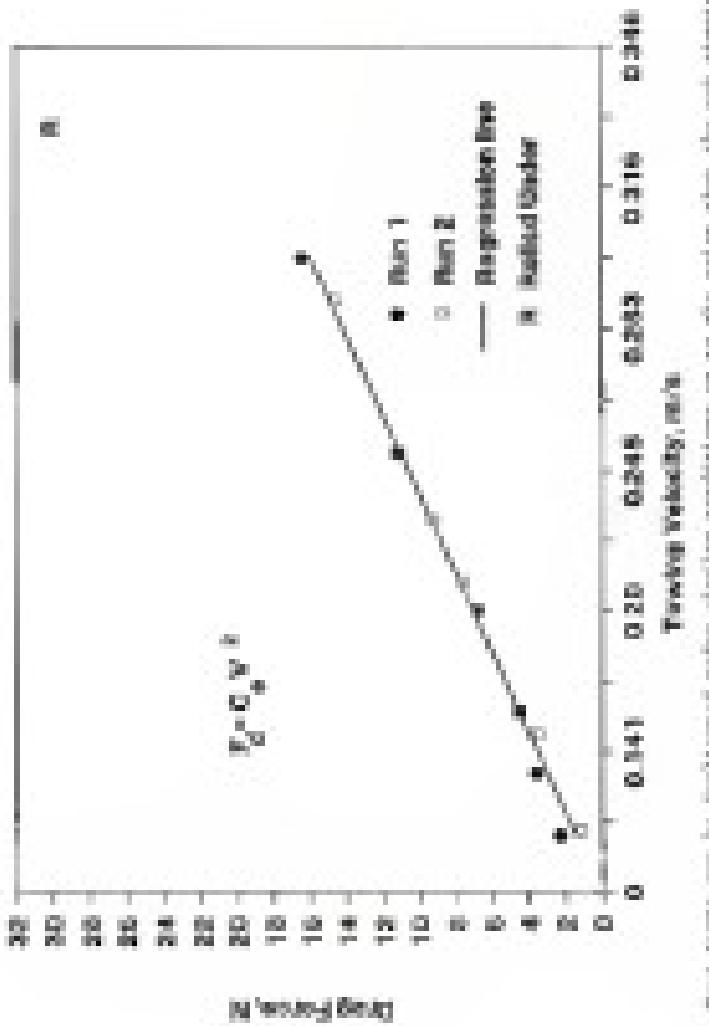


FIGURE 4.8. A "Bell curve" plot of the testing data from each panel under the same conditions. The yield increased and the portion of the area under the curve increased as the number of entries increased to 30 entries.



regression equation (Equation 4-9), the corresponding values of standard deviation (above or below) and error (above or below) were plotted (Figures 4-10 and 5-11). For the two turns analyzed by excluding data for the largest testing velocity, the ranges of the standard deviations fell in the range of the standard error ($\pm 10\%$ from zero level). The fact that the ranges of the estimated deviations of C_D at "wall shear" velocity did not fall between the ranges of the expected error shows that the flow behavior of the water hypodermic droplets near "wall-shear"

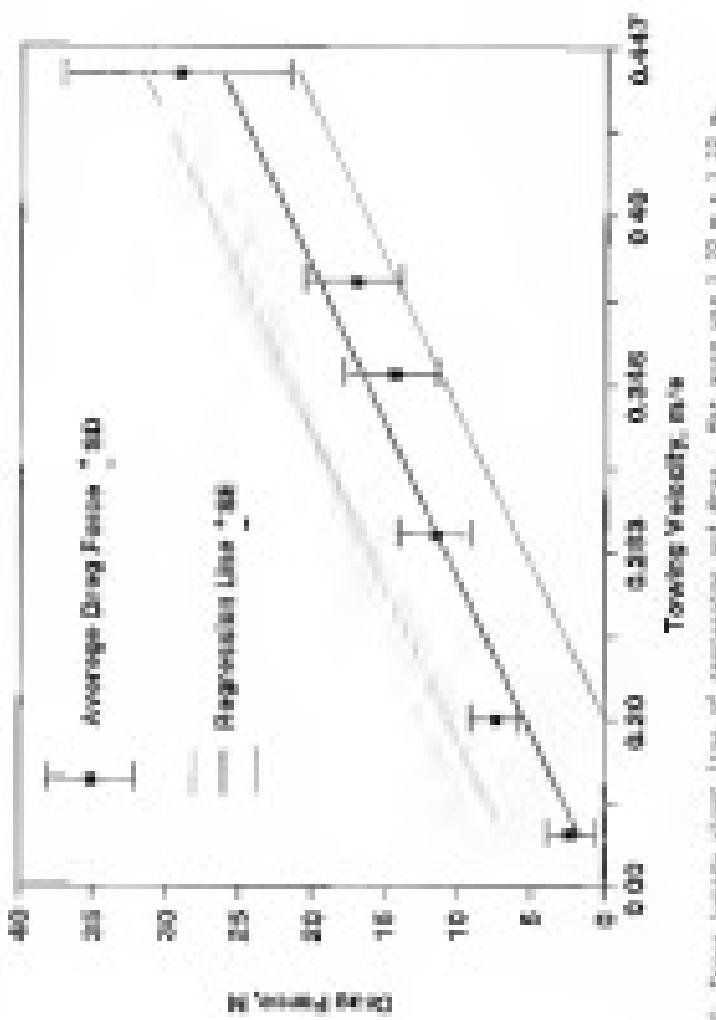
Values of proportionality factor, C_D (Equation 4-8) were computed and errors are represented in the next four Figures (Table 4-9). In the turns, percent relative error was greater than 10%, whenever the number of data points was less than five. Indeed, data analysis when one had enough became more complex because "wall-shear" very quickly. In these cases, the data points were scattered. However 85 percent of the C_D values had less than ten percent relative error. Error was not consistent in each experiment nor between experiments. Values of the coefficient also appeared to vary according to physical variations in the air and turbulent plume structure.

Conclusions

Provided that Equations 4-8 and 4-9 adequately describe drag for any choice of plume and air size and that C_D is not a function of Reynolds number, experimental drag coefficient should be easily given by

$$C_D = \frac{F_D}{\rho \cdot U_p^2 \cdot A_p} \quad (4-10)$$

where $F_D = 1 + \left(\frac{D}{L} \right)^2$



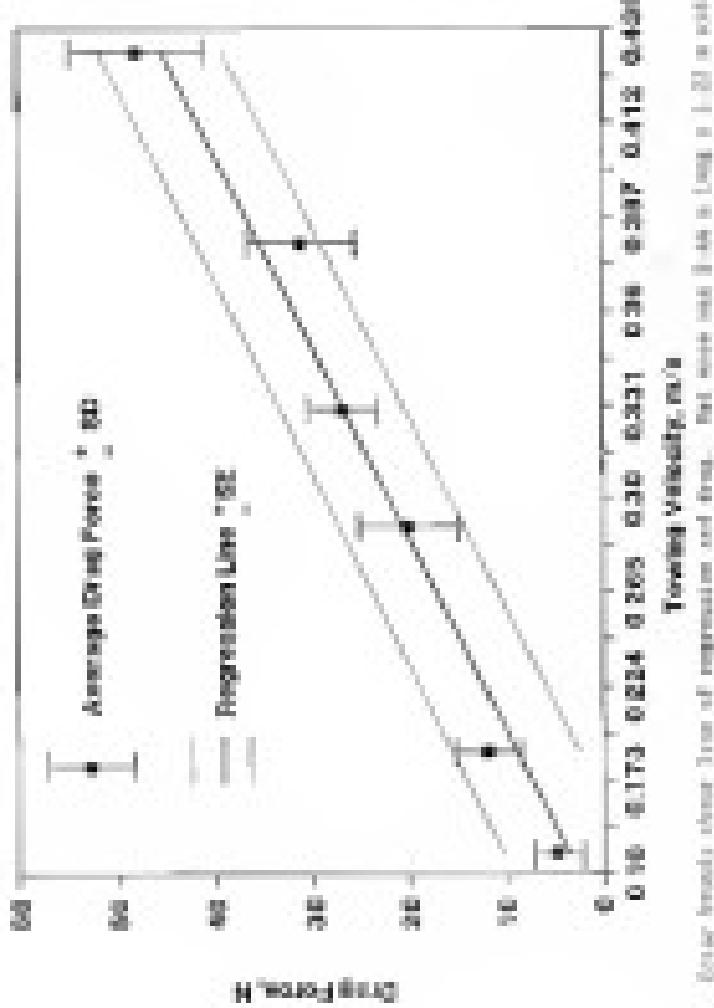


Figure 4.11: Four linear plots showing Total Vessel length vs. Dose (mg/ml) for each organ. Error bars represent standard error of the mean ($n = 3$) in each group.

Table 4.8 Coefficients for empirical equations that relate drug doses to velocity data.

$D_1 = C_1 \cdot v^2$	C ₁ & R ₁	Number of dose points	Sample size [log]	Sensitivity	Plane fitting
				$\log v^2$	
Experiments 1					
200-0	23.0	7	0.44 ± 0.132 ±	16.3	other 9011
200-0	20.0	7	1.33 ± 0.133 ±	15.9	
200-0	20.0	3	0.51 ± 0.133 ±	17.6	
200-0	6.00	8	1.33 ± 0.131 ±	17.9	
Experiments 2					
200-0	18.0	6	0.44 ± 0.133 ±	15.7	other 9011
200-0	20.0	6	1.33 ± 0.133 ±	20.3	
140-0	14.00	7	0.44 ± 0.133 ±	15.1	
70-0	15.0	4	1.33 ± 0.131 ±	22.1	
Experiments 3					
200-0	8.00	11	0.44 ± 0.137 ±	20.2	other 9011
200-0	6.00	7	1.33 ± 0.133 ±	20.0	
140-0	8.70	7	0.44 ± 0.133 ±	21.3	
70-0	3.00	11	1.33 ± 0.131 ±	21.3	
Experiments 4					
Experiments 4a	Milligrams				
200-0	17.0	7	0.44 ± 0.132 ±	other 9011	
200-0	20.0	6	1.33 ± 0.132 ±	15.6	
200-0	8.0	7	0.51 ± 0.132 ±	15.8	
Experiments 5					
200-0	6.00	12	0.44 ± 0.133 ±	15.8	other 9011
60-0	6.00	6	1.33 ± 0.133 ±	15.5	
120-0	6.00	6	0.44 ± 0.133 ±	15.8	
40-0	4.00	6	1.33 ± 0.131 ±	20.0	
Experiments 6					
100-0	14.0	12	0.44 ± 0.132 ±	17.6	other 9011
100-0	20.0	6	1.33 ± 0.132 ±	20.9	
100-0	20.0	6	0.44 ± 0.132 ±	20.9	
100-0	15.0	7	0.44 ± 0.132 ±	21.0	
90-0	9.00	11	1.33 ± 0.131 ±	14.9	
Experiments 7					
200-0	20.0	8	0.44 ± 0.132 ±	16.7	other 9011
200-0	20.0	8	1.33 ± 0.132 ±	16.4	
200-0	8.00	8	1.33 ± 0.132 ±	16.5	
200-0	20.0	8	0.44 ± 0.132 ±	16.9	

The values of C_p obtained by this expression from C_p^* are tabulated in Table 4-1 along with values of the slope factor and other variables. Variables t_{p_0} and t_{p_1} are calculated from expressions developed earlier (prior to Chapter 2). Since t_{p_0} is different to denote due to the presence of salts, but, since they have been shown to vary little within a population or by an animal model, the following substitution where no such short plastic have been measured, suffice: $t_{p_0} = t_{p_1}$ plus 30 sec.

To assess the accuracy of the experimental drug coefficient, experimental error of Equation 4 is now calculated by the following equation (in 1971 (Bentley 1979)):

$$\delta_{C_p} = \sqrt{\left(\frac{\partial C_p}{\partial C_p^*} + \frac{\partial C_p}{\partial t_{p_0}} + \frac{\partial C_p}{\partial t_{p_1}} + \frac{\partial C_p}{\partial k_{p_0}} + \frac{\partial C_p}{\partial k_{p_1}} \right)^2} \quad (4-10)$$

where δ_{C_p} denotes the present error in the drug coefficient, contributed by the 5-th parameter. The error for each estimated value of C_p is shown in Table 4-2, and the average is approximately 10.0 percent of the mean. δ_{C_p} would vary with the quality of water (Minx et al. 1971) and temperature, but that error was assumed to be within 0.5 percent. The errors due to the estimation of t_{p_0} and t_{p_1} were assumed to be 12% and 2% respectively since that is approximately the error introduced by the corrected expressions of Chapter 2. The error due to k_{p_0} was estimated to be 10%. After substitution of these values of factors error from the error equation, the percent experimental error for the drug coefficient was 15%

Table 4.1 Drug metabolism for different anti-gastric and other
new characteristics

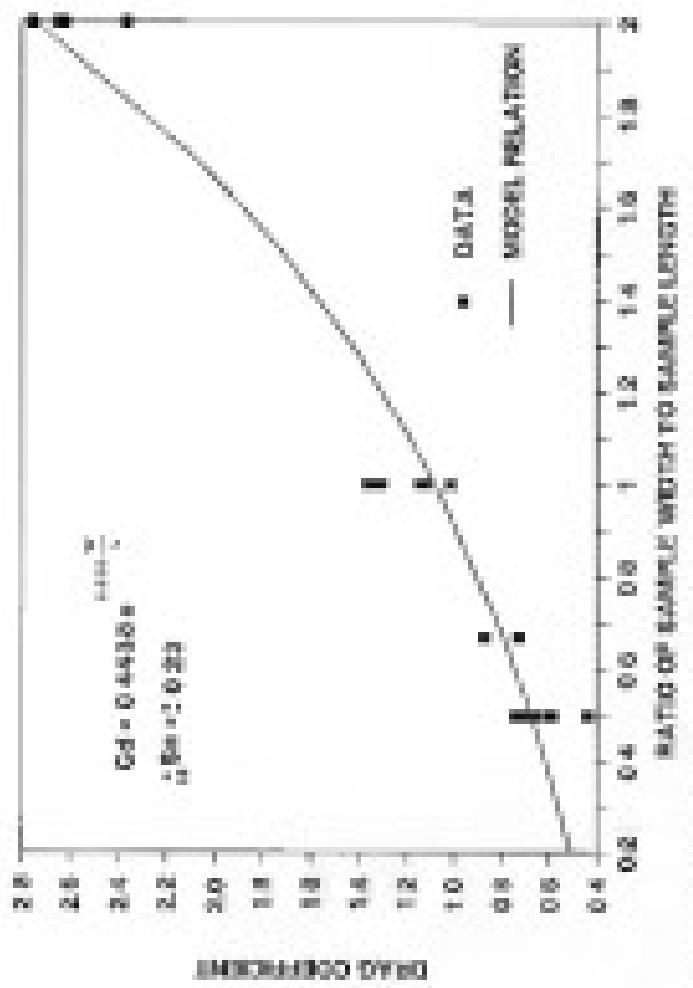
$\frac{C_{\text{D}}}{C_{\text{P}}}$	$\frac{C_{\text{D}}}{C_{\text{P}}}$	Initial esterase activity of plasma sample	Time to maximal hydrolysis	Reaction rate constant k_2	$\frac{C_{\text{D}}}{C_{\text{P}}}$
Experiment 1					
262 D	0.73	520	0.5	0.00-0	
262 D	1.23	240	1.0	0.00-0	
149 T	1.13	120	0.5	0.00-0	High
Experiment 2					
262 D	0.61	320	0.5	0.00-0	
262 D	1.13	120	1.0	0.00-0	
149 D	1.10	120	0.5	0.00-0	Low
76 D	0.59	120	0.5	0.00-0	
Experiment 3					
262 D	0.60	320	0.5	0.00-0	
262 D	1.20	120	1.0	0.00-0	
149 D	1.10	120	0.5	0.00-0	Low
76 D	0.59	120	0.5	0.00-0	
Experiment 4					
262 D	0.60	140	1.0	0.00-0	0.00-0
149 D	1.10	80	2.0	0.00-0	
76 D	0.61	80	0.5	0.00-0	
Experiment 5					
262 D	0.74	640	0.5	0.00-0	
149 T	1.12	320	1.0	0.00-0	
149 T	2.00	120	0.5	0.00-0	
76 D	0.60	120	0.5	0.00-0	High
Experiment 6					
262 D	0.60	70	0.5	0.00-0	
149 D	0.47	50	0.47	0.00-0	
149 D	1.30	10	1.0	0.00-0	
149 T	0.37	10	0.37	0.00-0	
149 T	1.00	10	0.5	0.00-0	High
Experiment 7					
262 D	0.73	120	0.5	0.00-0	
262 D	0.73	120	0.47	0.00-0	
262 D	1.00	10	1.0	0.00-0	
149 D	0.50	10	0.50	0.00-0	

The functional relation between C_d and shape factor $\frac{S}{L}$ was determined. A regression equation relating C_d to $\frac{S}{L}$ was determined in the next issue [Figure 4-12], and is as follows:

$$C_d = 0.462 \exp(0.320 \frac{S}{L}) \quad (4-12)$$

For all values of C_d , were used in the formulation of this relation because they were deemed to be outside of 95 percent confidence intervals. Confidence intervals of C_d were calculated for every shape factor $\frac{S}{L}$. The outliers were marked by four groups of six values. Two outliers originated from an area of 0.06 long x 0.04 wide, and two outliers from an area 0.01 x by 1.00 m. The former values were results of systematic error. They were related to errors resulting to the lowest drag values. As was mentioned before, these last losses may have been recorded incorrectly by the data logger. The highest value of the data logger added to the value of the rest five. The attempts were made to correct this problem. One method, which resulted in reasonable values of C_d (Tables 3 and 3b), was to multiply the data signed from the static paper positioned on the container base. The second method, which consisted applying a higher voltage to the strain gauge, was unsuccessful.

On the other hand, the two outliers from the other sample who could not be explained until after withdraw of the experiments were removed. The plates were now leading to the flow. The leading action forced friction drag over pressure drag, therefore the drag coefficients were lower than reported (Goldschmid, 1970).



The regression equation relating C_d to R_b was analysed for experimental and dispersion errors. Dispersion of the data around the regression curve is given by S_e , where e is the t statistic (95% confidence level) and S_e is the standard error due to the regression if the regression equation is valid, the confidence intervals of C_d for every slope factor $\frac{R}{L}$ should fall within the range of C_d given by the standard error of the regression. This was true except for R/L equal 2 (Figure 4.10). However, the discrepancy is negligible.

Summary...

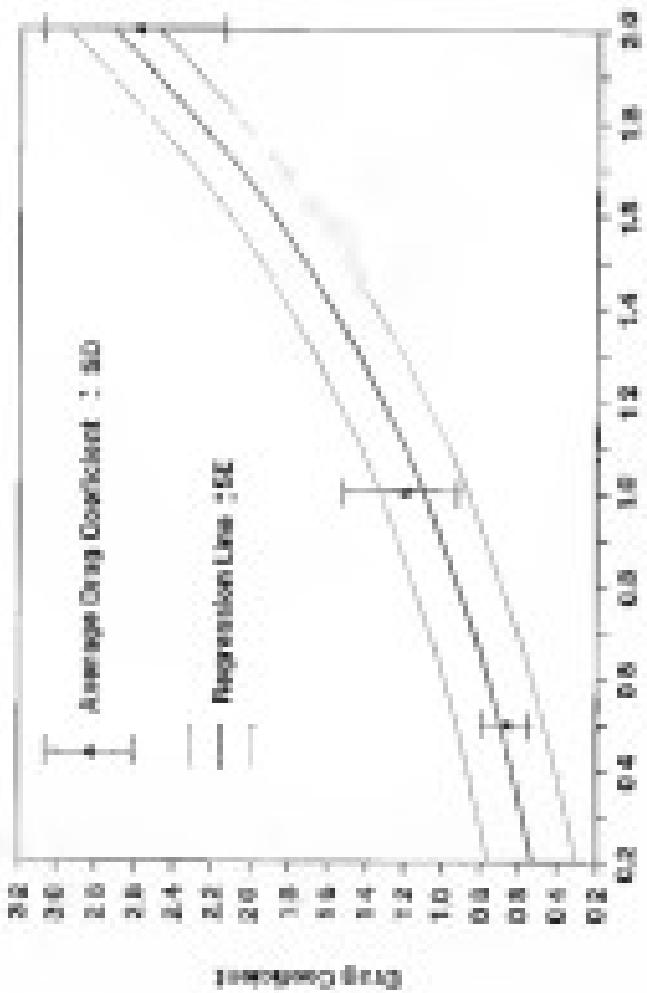
-Hydrodynamic drag coefficient of water flowing in turbulent at Reynolds number has dependence on the slope of L is not
 -The water flowdrag equation was based on Review of drag formulae and to include the drag coefficient, projected area and number of ribbons and the square of velocity.

Model Generalization...

Model determines of water hydraulic losses when an external force applied on a soil suspension plait against wall after. Basis of the plait is against static compression have been observed to be the ribbons. The objective of this section is to determine
 -especially the kinetic energy and frictional ratio of the ribbon.
 Literature Review.

Annotated study of the ribbon results that it is a composite material of one or more composite having various, mainly nonlinear physical and mechanical properties. Water contact increase, the other material composition such as tensile of plaites or layers of particle,

Figure 4.12: Mean female mean loss of inhibition and drug concentration



been reported elsewhere (Thompson and Kinsella, 1981).

Reinforcement by the filament consists of fine regular outer regions covered with profusely-branched roots of 0.4 to 1.5 mm diameter, moderate, vertical, irregular scale-like. Between very low length fractions approximately 1 to 20 cm and 100 cm or more there apparently are no shear similarities.

Electro properties are assumed to vary principally in the radial direction because of the asymmetric shape and radially varying sequence of phases. The electric properties of a core sub-area similarly characterized and have been extensively studied (Kinsella, 1980; Kinsella and Chikwele, 1982), and Chikwele and Kinsella (1984). The methods and equations these investigators used were adapted to this work to determine the electric response of outer layers due to motion of their epidermal rhizomes.

Equations for the determination of elasticity modulus, E , and Poisson's ratio, ν , of elements under quasi-static radial compression between two moist soil plates using a large deformation approximation are (Chikwele and Kinsella, 1984)

$$\frac{E}{\rho} = \frac{2(1-\nu^2)(2M^2 - 3M + 37\nu^2\cos^{-1}(M)\sqrt{2M})}{(1-\nu^2)(1+3\nu^2\cos^2(M))} \quad (4.10)$$

where M is the force experimentally applied force per unit length of the cylinder. Poisson's ratio, ν , is unchanged

$$\nu = \frac{0.000016 \ln(\mu/\lambda) - 0.000016 \ln(\lambda) + 0.000016 \ln(\mu) + 0.000016}{\ln(\mu) - 1 + \mu\lambda + \lambda(\ln(\mu)/\ln(\lambda))} \quad (4.11)$$

- e. In the determination of the diameter, width and height of the cylinder.
 - f. In the normal approach of the FEM plate to the center of the cylinder, or half of the total stored deflection of the cylinder.
- $k = \tan(\theta) < 1$
- T is the half-angle of measure between the cylinder and the flat plane which can be calculated by the following equation:

$$\tan T = \frac{(k_1 k_2 - k_1^2 + k_2^2) \alpha - k_1^2 \alpha \cos^2 \theta - k_2^2 \alpha \sin^2 \theta}{(k_1^2 + k_2^2 + k_1^2 k_2^2) \alpha + k_1^2 k_2^2 \sin^2 \theta}$$

where $\alpha = \pi/2$ radian of the cylinder.

$$k_1 = \frac{\sqrt{1 - k_2^2 \alpha^2}}{k_2 (1 - k_2^2 \alpha^2 + k_1^2 \alpha^2)} \quad \text{and}$$

$$k_2 = \frac{\sqrt{1 - k_1^2 \alpha^2}}{k_1 (1 - k_2^2 \alpha^2 + k_1^2 \alpha^2)}$$

Values from these equations which are based on a large deformation theory, must compared to values based on the linear Hooke elastic constant theory and it was concluded that the large deformation theory has the most numbers that apply by one person.

When two cylindrical sections, with their axes parallel, are pressed in contact by force F per unit length, they will contract over a long strip of width b lying parallel to the axis. To determine the value of α and used dimensional compression, Hooke contact theory can be used. The assumptions made in the Hooke theory are as follows (Johnson, 1989): [1] The surfaces are continuous and non-overlapping; [2] $0 < 1 + \mu < \alpha$ the radius of the contacting body; [3] The strains are small; [4] $E_1 = E_2$. By such might can be considered

to no discrete hair-sprout at 0.1...0.2 length of the epidermis, and (b) the surfaces are frictionless.

For the rhizome problem, the First and third conditions are met. Johnson (1980) states that the Betti theory is valid for non-composite materials. For a stack of natties where each could have surface layers whose elastic properties differ from layer layers, Betti theory is valid as long as the top layer is large compared with the bottom one (id.). Also, rhizomes are composed of material similarly to that in natties, and the Betti contact theory predicted correctly the value for α (Gholamzadeh and Amini, 1994). Therefore, the use of the Betti contact theory instead of a large deformation theory to predict nattie compression probably would not lead to significant errors. Finally, although the rhizome is covered with rhizoids could cause friction between neighboring rhizomes, Betti theory can be used for easier speculate because friction at the interface of the two neighboring bodies brings two small contact places a part only if the elastic constants of the two materials are different (Johnson, 1980). Therefore, the Betti contact theory can be used for nattie speculate, and the following formula for total compression, δ , of a rhizome diameter compressed between two other hydroids through the mid-plane of the nattie may be used:

$$\delta = \frac{2\pi(1-\nu_1^2)}{E_1} \left[\ln \left(\frac{1}{2} \pi R_1 \right) + \ln \left(\frac{1}{2} \pi R_2 \right) - 1 \right] \quad (10)$$

where, R_1 are the non-frictionless-free Betti contact theory given by:

$$\sigma^2 = 0.02 \times 10^{-12}$$

where σ^2 is the square of the true compressive stiffness or

$$\sigma^2 = \frac{1-\rho^2}{\rho_1^2} + \frac{1-\rho^2}{\rho_2^2}$$

Materials and Methods.

In this section the procedures used to determine Teng's modulus and Paluszak's model for wave lysmuth rheology are discussed. These values will be used whenever the total compressive response of a rhizome is tested (Question 4.20).

Rhizomes were collected from Rano Lake, a lake where wave lysmuths are not harvested consequently, the collected wave lysmuth rhizomes were collected so that long root stumps could be cut. Stumps were 2 to 3 cm long and 1 to 3 cm in diameter because these dimensions are typically prepared for consumption here (Ridder, 1982). Roots were left on the rhizome to differentiate it as natural compression medium. After the samples were cut, rhizomes were observed to be rounded and covered with a soft, spongy tissue which probably represented interradial material. The interradial area produces the roots and leaves. The nature of spongy material varied from sample to sample. The samples were subjected to radial compression in an Instron universal testing machine until the rhizome cracked. Loading rates of 50, 100 and 200 N/sec were used to check stress-strain behaviour.

Typical force-deformation behaviour of a radially compressed rhizome specimen exhibits two distinct stages. During the first loading

of the test, the core and sponge material fracture with very little resistance. This behaviour is similar to that shown by a linear elastic isotropic shear modulus resistance to initial deformation in the process beneath the sponge material. Unlike many biological materials, sponges do not have a low-yield point, although a definitive rupture point was evident.

Tseng's modulus was calculated from Equation 4-19 with values of deformation and Poisson ratio from the linear portion of the stress-strain curve. Poisson's ratio was calculated by the method of Gulland and Linsley (1961) and the experimental setup is briefly described below.

Successive photographs of the cross-section of each sponge sample were taken with a video camera during radial compression. The test ended when the sample cracked. The video was viewed frame by frame and the total lateral extension and axial compression were measured. These measurements were used to calculate Poisson's ratio by the use of Equation 4-19.
Results and Discussion.

Poison's ratio was successfully determined for two sponge samples other samples tested to failure under the flat plane which, had measurements difficult with video analysis. These two values are 0.46 and 0.49. They are considered typical for a spherical cell approximation of a liquid (Kleinstreuer, 1990). The extremes are 10 percent more, therefore these values are accepted. Since the average value, 0.47, was used in the calculation of Tseng's modulus

The Tseng's modulus ranged from 0.5 to 31.8 GPa with an average of 6.9 GPa (Table 4-2). Loading rate appeared to have no effect on these values. The angularity of the surface was known phase for core and

(10 MPa) and Elmer's toothpaste (6.4 MPa) (Delivette and Souza (1994), and Souza et al., 1999).

Further, the force per length of fibres at the transition point of the force-displacement curve ranged from 30 to 40 MPa. As no mandibular molar, this force corresponds to the force required to flatten the teeth against the fibres. From the data, the mean apparently projected 0.5 cm apart from the fibres.

4.3.4. Summary

Pelican's ratio was determined to be 0.42 and the apparent Young's modulus 8.0 MPa. The force required to flatten teeth against a fibres is between 30 and 40 Newton per meter of fibres length. In addition we proposed that model predict the temperature increase induced when fibres contact.

Table 4.8: Experimental values of apparent Young's modulus of major lymphatic fibres

Leading velocity cm/s	Young's modulus MPa
00	10.0
02	8.0
04	6.1
06	11.0
08	10.0
10	7.0
12	10.1
14	8.7
16	10.0
Average	8.9
Standard deviation	1.1

CHAPTER 6 MATHEMATICAL MODEL OF ROLLING BUCKLING TEST

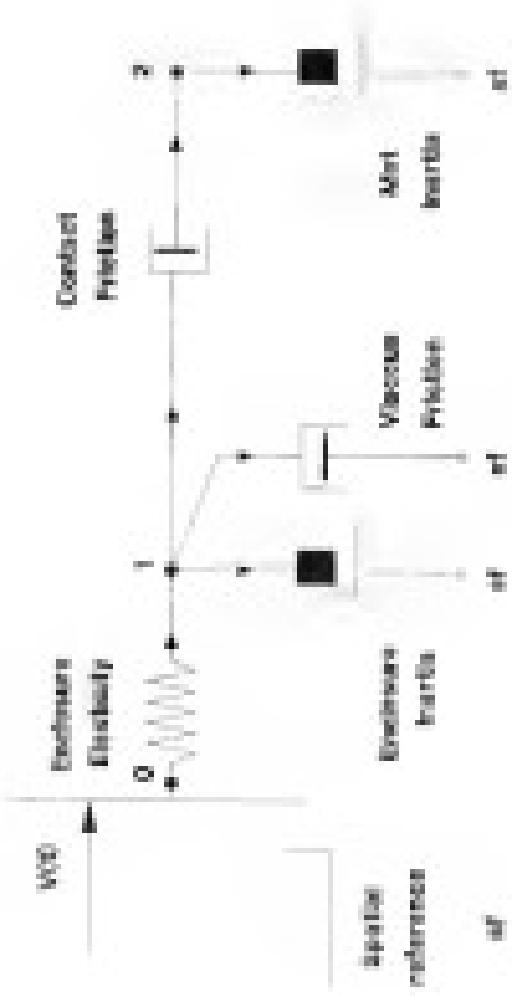
This chapter is concerned with development of a mathematical model of a roller buckling testing system. It is based on physical laws that govern the system and their interconnections. Key principal system variables are car horizontal velocity and displacement. Using testing velocity as the input, the model was solved numerically, and calculated values of testing force and car separation were compared to experimental data. Further, in this chapter the physical condition of the system prior to vertical displacement or "roll over" is described.

General Model

To construct an accurate mathematical description of components and their interconnections, knowledge of the structure of the system is required. At the beginning of this study, physical components of a roller buckling test and their interconnections were obscure and ill-defined. In the process of defining component interconnections we manipulated until the resulting mathematical model generated necessary input-output relationships for different boundary and initial conditions. The resulting conceptual model of a mechanical system has features of a roller buckling test of tenth I, and tenth II and is shown in sequence from top to bottom in Figure 6-6. The distance from

Mechanical System

Figure 1.10: Mechanical system of a sensor based on two parallel beams.



rule 1 to rule 3) is the length l_1 of the soft layer, rule 1 represents the case of a soft soil rule 2, the hard soil.

The measuring device is analogous to the one equipment used in the testing experience described in Chapter 4.8. For other testing devices different kinds and arrangement of elements could model the measure. It is represented by a spring and a mass element. The spring (representing the oscillation base in the testing tool) appears to be connected to a frame of three more rigid elements in total.

The outer hypothesis are represented by two types of friction elements and a mass element. One of the friction elements is associated with viscous friction of the outer hypothesis moving through the soil. As the speed begins to move motion is impeded first in the rear. It is for this reason that the viscous friction element is positioned at rule 1. The other, connected between rules 1 and 2, is associated with sliding friction of contacting particles.

Initially the spring is at rest. At time $t > 0$, the oscillation starts at velocity $v_0(t)$. As the soil begins to move, it impacts these respective rules, the soil moves uniformly at the speed of the measuring device.

Element Descriptions.

Mass element. To represent part of a physical system by an idealized coordinate mass, it is necessary that all of the particles be connected together and that they move with substantially equal or proportional velocities and accelerations (Bauer et al., 1971). The corresponding equation for an ideal mass element is derived from Hooke's second law and inertia force F_0 equals

$$F_{B_1} = B_1 \frac{\partial f}{\partial t} \quad (6.10)$$

where, t denotes time number

$$B_1 = \text{sum of the volumes} + B_0$$

B_0 = sum of water displaced and plus sum of water associated with the net $\equiv B_{01}$

V_1 = Velocity at most significant net volume t with respect to original reference.

Water mass is added to that of the most significant net, because it gives the additional force required to maintain the sum of fluid net in action by the introduction of the gross part submerged in the water. The additional amount of water mass depends on the slope of the moving object [Herbert and Gross, 1965]. These hydrostatic stresses, the principal objects in flow, are similar to circular cylinders. For circular cylinders in unidirectional flow, the apparent pressure is zero due to water against the sum of fluid displaced by the circular cylinder [Prandtl and Tietjens, 1934].

Rectangular, irregular and curved shapes, their resistance is always in shape often subjected to a force can be characterized by a different element, provided that an algebraic relationship exists between the configuration or configuration and the force. The rectangular form used in the testing experiments can be modified to non-uniform shapes. For a linear condition, Heriot gave R_h equals

$$R_h = K \cdot R_{01} \quad (6.11)$$

where K is an elastic constant and R_{01} equals the non-linear deflection that is given by:

$$X_{\text{eff}} := X_0 - X_1$$

where X_0 is the displacement of the cutting device and X_1 is the displacement of a water specimen made of $\beta = 0.001$, the error deflection is defined to be zero, with $X_0 = X_1 = 0$, and we have applied to the oscillation.

For small deflections X_1 of a specimen base, it can be calculated as follows:

$$X_1 = \frac{P}{E} = \frac{\Delta A}{L} \quad (16.2)$$

Here, P = concentrated load at free end of base,

L = base length,

E = modulus of elasticity,

$$P = \text{weight of beam} = \frac{\rho g b h^3}{3}$$

b = base width, and

h = base thickness.

For $b = 0.05$ m., $h = 0.0105$ m., $L = 0.30$ m., and X (free end) = 2.07×10^{-7} m., E = 3,000 N/m. The experimental cutting device had four additional beam having respectively the theoretical value of X_1 is 23.000 N/m..

Another useful property of the modified oscillation device is its natural frequency, ω_n , is mathematically

$$\omega_n = \left(\frac{E}{\rho g} \right)^{1/4} \quad (16.3)$$

The period of vibration of a undamped oscillating device depends on the mass of oscillating device M_0 , and on κ of the oscillating beam, and is independent of magnitude of amplitude. For an oscillator mass equal to 100 kg, and κ equal to 12 000 N/m, the system natural frequency is 5 Hz.

Steady-state. Drag friction is primarily due to the presence of water boundary regions. Of, in addition, the oscillating device is equipped with objects that protrude from the water, different types of resistance elements may have to be applied to the model. In chapter 4, the description of drag due to resistance was found to be a nonlinear function of net velocity. In the emulsified model, net velocity is the velocity of water. Therefore, the steady-flow description of the resistance element is

$$R_f = 1000 R_p \delta_p \delta_M |V_1| |V_2| \quad (30-4)$$

The absolute value symbol is required to change direction of flow with change in direction of velocity. Drag coefficient for steady flow was found to be $C_d = 0.008 \exp(0.008 \frac{|V|}{U})$ (Equation 4-26).

Drag will not be exactly this as was determined experimentally because several physical responses to the waves were simultaneously added (e.g. inertia, drag, and corner friction elements), and drag coefficient is not linear. When a fluid body starting from rest moves through a fluid, water and resistance does not change with time. Viscous drag is proportional to the rate of increase of energy resulting from the lengthening of these waves (Putter 1984). Also the drag coefficient of a group of arbitrarily shaped particles varies

with concentration in a turbulent fluid was found to be function of particle Reynolds number and degree of turbulence. In a set of the measurements (Elter and Ross 1980 and Tsvetko and Gerasimov 1984) therefore, the value of C_D in steady flow can not be used in equation 3-6 while calculating, because the existing experiment and theory described in the previous chapter give a precise mathematical description of drag only when the ratio of increase of water to constant α and the effect of system acceleration are absent. Therefore, no expression for C_D valid for steady flow conditions was obtained through model simulation.

Friction Factor Another friction factor arises between plants and is connected to each other through interactions of particles, a condition that occurs when the population density is high and when the plants are healthy and have not been attacked by insects or disease. Index of particle interaction is measured by compacting (see Chapter 3). When compacted particles stick against each other during relative motion, friction reduces their motion so a more loosely packed composite particles slide over each other. A pair of liquid agents between these particles become thinner and thinner. If the resulting fluid flow is faster, the friction should be constant, which forms F_2 :

$$F_2 = R_{21} F_{21} \quad (3-7)$$

where $R_{21} = N_1 \times N_2$, variables N_1 and N_2 correspond respectively to horizontal velocities of the front and back ends of a plant

Symbolic and with respect to a spatial reference, and R_1 is the hydrostatic plant friction coefficient, which is proportional to the current area and the water viscosity and inversely proportional to the thickness of the film (Cline and Peltola, 1979). Parameter R_1 was evaluated through simulations.

Model Formulation.

In this section, a set of equations were developed that described a wave breaking wave system. First, state equations are written down, a set of state variable equations is derived, and finally, a set of output equations is derived from the state variables.

State equations. The system model has two nodes; therefore, two initial equations describe force interactions at these nodes. Applying Bernoulli's law, the sum of the forces at these nodes were equal zero. The state equations are:

STATE	EQUATION	(n)
1	$-F_x + F_{R_1} + F_y + F_z = 0$	(n-6)
2	$-F_x + F_{R_2} = 0$	(n-7)

State-variable equations. A set of state variables describes completely the behavior of a system in response to the inputs. Therefore, knowledge of their values at reference time t_0 , and the values of the inputs for all $t > t_0$ is sufficient for evaluation of state and control variables of the system for all $t > t_0$ (Cline and Peltola, 1979). One state variable is usually associated with each energy storage element (e.g. inertia and spring elements)—for this study the state variables are R_{max} , R_1 and R_2 . The former parameter is small-break deflection and variables N_1 and N_2 correspond respectively to horizontal translation of the front and

both ends of a minor hydraulics line with respect to a spatial reference. To derive state variable equations, an expression for the derivative of each state variable is formulated in terms of other state variables, flow, and inputs by using initial equations and component descriptions. The description used for the rate of change of R_{p} , is derived below, and it is in terms of input variable V_1 and state variable V_2 .

$$\frac{dR_{\text{p}}}{dt} = \frac{dR_{\text{p}}(V_1, V_2)}{dV_1} = V_2 - V_1. \quad (5-10)$$

The state variable equation for V_1 was derived in the following way. First, the time derivative of V_1 was expressed by using the component description of the kinetics of the resistance device (Equation 5-8):

$$\frac{dV_1}{dt} = R_{\text{p}}/R_{\text{v}}. \quad (5-11)$$

Next, another expression for R_{p} was found since a state variable equation is to be given of other state variables. It was obtained from initial equation 5-9, and substituted into Equation 5-11.

$$\frac{dV_1}{dt} = R_{\text{p}} - R_{\text{p}}/R_{\text{v}}. \quad (5-12)$$

Finally, the second state variable equation was derived after appropriate component descriptions were substituted from Equation 5-10:

$$\frac{dV_1}{dt} = [R_{\text{p}}(V_1) - (2000.0 R_{\text{p}} A_{\text{p}} C_{\text{p}} V_1 R_1) + R_{\text{p}}(V_1 - V_2)/2R_{\text{v}}] \quad (5-13)$$

The first state variable equation (Equation 5-10) was derived in a similar fashion:

$$\frac{d^2\theta}{dt^2} = T_{R_0}/R_0 = T_g/R_0 \\ = \{B_1(1/X_1 - R_0/1)\}/R_0 \quad (5-10)$$

Damping equations. The damping, rolling force, rolling moment, force and drag force are output variables of the model of interest, and they are calculated at any time t . From the state variables, rolling contact and drag force are computed from Equations 5-4 and 5-5. The computation for a car of original length L is computed from:

$$\text{For suspension } \approx L = (X_0 + X_1) \quad (5-11)$$

where X_0 and X_1 are calculated after the following equations are integrated:

$$\frac{dx_0}{dt} = X_1 \quad (5-12)$$

$$\frac{dx_1}{dt} = X_2 \quad (5-13)$$

Rolling force is calculated

$$\text{Rolling force} = C_1(X_0, 1) \quad (5-14)$$

Method of Reduction and Parameter Estimation

Equations 5-8, 5-11 and 5-12 are a system of coupled three-order differential equations, one of which is nonlinear. They represent a non-holonomic rolling system. The system was solved in successive steps to reduce the rolling force, study variations of "roll over" and the suspension and damping parameters C_1 of constant fiber constraint and R_0 .

To solve the equations, initial conditions, estimates of parameters, T_0 as a function of time, and numerical integration were needed. System variables and outputs were evaluated numerically because one of the equations was nonlinear and two parameters were unknown. The initial conditions were used to compute initial and some output variables. At t_0 , T_0 , P_0 , and X_0 were always zero or zero. Displacement of the spring element was zero ($X_0 = X_1 = 0$) and X_2 equaled 1. Further, at t_0 , velocity T_0 was a function of time and system variables and outputs were evaluated sequentially until the system reached steady state or until the derivatives of the mass variables were zero.

The fourth-order Runge-Kutta formula was used to solve the equations in a "problem solver" program written in Turbo-Pascal (Appendix A). The program utilized a published version of Runge-Kutta (Press et al., 1988). The step size was based on accuracy, and 0.02 seconds for an integration interval of 10 seconds.

To estimate the parameters R_1 and C_1 , the following scheme was adopted. The program accepted estimates for parameters R_1 and C_1 , solved the differential equations, and selected values of testing force and rate (stepwise or specified time). Output values were compared to experimental values that were measured during the unsteady part of the testing experiments discussed in Chapter 4. The possibility for parameter nonlinearity was rejected until comparison between calculated and experimental data was better than 80%. Testing forces at rates 1.14 times described in testing experiments 4 through 8 [Table 4.6] were calculated. Plots of the initial staged free decay model fit to

Initial evaluations of parameters, C_0 and R_0 , were based on some prior information. For instance, the drug coefficient (α) initially this coefficient was assumed to equal the experimental value for steady state conditions at dosage 0. The other parameter, R_0 , was thought to be constant because physical factors that could affect it do not vary appreciably in a water capsule cell. Prior to coefficient, R_0 , is known to be proportional to capsule area and inversely proportional to film thickness. In water lyophilized, film thickness and viscosity do not vary and capsule area probably varies slightly as a function.

Results of Parameter Estimation and Model Simulation

Parameterizing. Correlation coefficients between initial and experimental data for six different conditions ranged from 0.80 to 0.96 (Table 5-8). The lower coefficients were associated with experimental and calculated resting force measured at $t = 0$ hr. This was partly due to uncertainty of the user time a specimen started (i.e., the beginning of an experiment was defined to be the time of the first measure data point recorded by the data logger). Also, the few data points were obtained because experimental data were recorded every second (i.e., large sampling time).

Equations describing R_0 and C_0 selected on the basis of these correlations (Table 5-9) were found to depend on plant size and other physical characteristics of a cell. Further, drug coefficient was found to vary with R_0 until steady conditions were reached, and it is described by the following equation:

$$C_D = \phi \tan \exp(\phi \tan \frac{\theta}{L}) \quad (5.26)$$

where

$$B_1 = B_2 = B_3 \quad \text{for } V_0 < V_a$$

$$B_1 = 1 \quad \text{for } V_0 = V_a$$

where B_1 is a constant. This expression supports the assumption that a test aircraft maintains speed when both the front and back winds are at the same velocity, and the steady drag coefficient of a test changes to its steady flow value.

Table 5.2 Correlation coefficient between model and experimental trailing force

Model size $L \times R$	Testing velocity m/s	Number of data points	r^2
1.00 m \times 1.00 m	0.80	0	0.99
1.00 m \times 1.00 m	0.74	0	0.99
1.00 m \times 1.00 m	0.71	0	0.99
0.60 m \times 1.00 m	0.70	0	0.99
1.20 m \times 1.20 m	0.68	0	0.99
0.60 m \times 1.00 m	0.70	0	0.97

The constant B_1 was necessary because without it, B_2 would not have a finite value at small values of X_0 . Constants B_1 were determined to be 0.5 for those of the experiments modelled, and to equal 0.25 for a case that was comprised of large planes and large distances with long runs (Table 5.1). The experiment number then represents the testing experiment modelled. This constant determines the value of the drag coefficient just as a unit starts from zero. As the value of B_1 decreases to different types of tests, the drag

condition increases. Therefore, it is conceivable that very low tensile and large stresses would affect it. In addition, R_p was not affected by soil geometry or tilling velocity.

Table 5.1. Values of R_p and R_t of the soils tested

R_p	R_t	Experiment number	Average plant length, cm
300.0	0.50	S	20.0
310.0	0.48	S	31.0
370.0	0.50	T	23.0
340.0	0.30	S	90.0

The values of R_p determined for each soil tested is presented in Table 5.1. It was affected by the type and condition of the plants that respectively are, but it was unaffected by soil geometry or tilling speed. The fact with the lowest value of R_p (30) was surprised at very small plants. The friction factor of each of larger plants varied from 340 to 370. This range of values could be due to anisotropy. Small plants were connected primarily by nodules and larger plants were connected primarily by overlapping leaves.

Effect of tilling velocity. Rate was conducted experimentally in one of two ways. For tilling velocities less than 0.80 m/s, the rate function is related to a top velocity input, and for tilling velocities > 0.80 m/s, the rate was measured incrementally to effect possible detrimental tillage effects on the tilling implement.

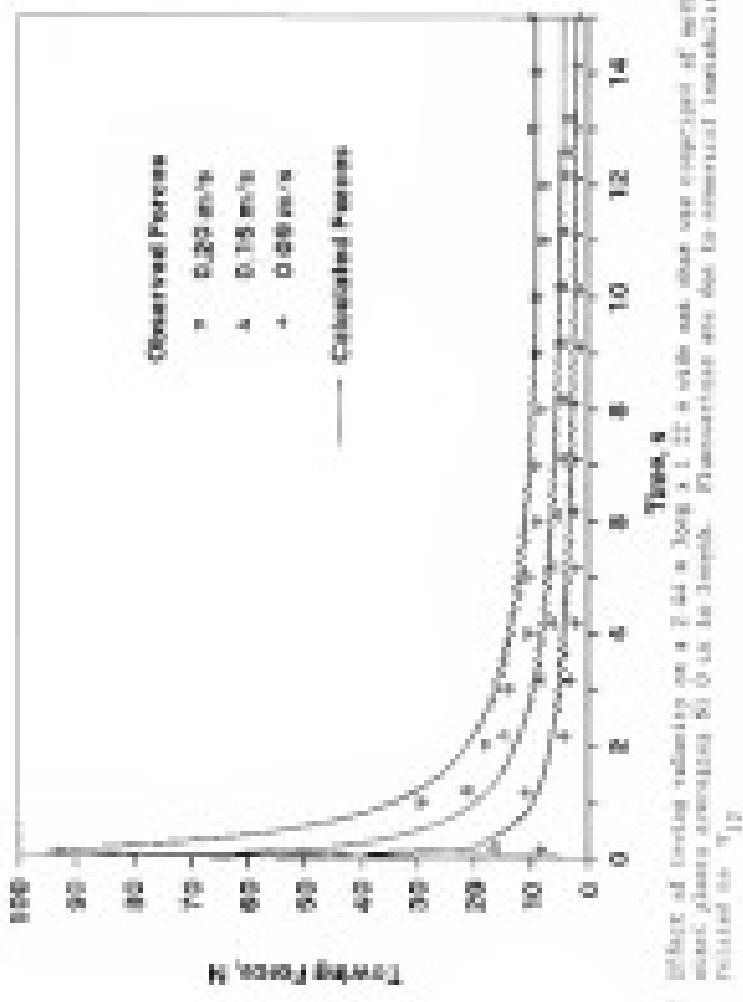
The following describes initial response to a single step velocity input. When it was used, spring force increased very quickly and then decreased gradually to a steady value. The maximum value occurred independently of test speed at approximately 0.1 n. However decrease due to the mass of the enclosure to the principal active component as the system up to that time which was expected because the natural frequency of the spring - enclosure system was 5 Hz. As speed increased and test length increased, the test longer the force to decrease to a steady value (Figures 5.1 and 5.2). For a sample size 2.44 m long by 1.33 m wide, time to reach steady condition ranged from 2 s for a velocity of 0.0 m/s to 10 s for a velocity of 0.8 m/s.

Sliding friction (Figure 5.4) and viscous friction (Figure 5.5) are responsible for damping (Figure 5.3). Viscous friction increases as the square of velocity, i.e., as sliding velocity increases its quickly becomes the principal active component.

The damping as measured at steady state, increased linearly with testing velocity. When R_1 equals 30, damping of a 2.44 m long by 1.33 m wide was increased with testing velocity according to this expression:

$$\text{damping} = 1.0 + R_1$$

To allow the effects of inertia of the enclosure, tests were conducted incrementally whenever the desired final testing velocity was greater than 0.25 m/s. Enclosure tested according to the description of the test rig experiments. Velocity inputs to the problem added and changed incrementally with time in an attempt to obtain these experiments. The velocities were done one hundred in total with very small steps; the other involved motion-control plants typical of a



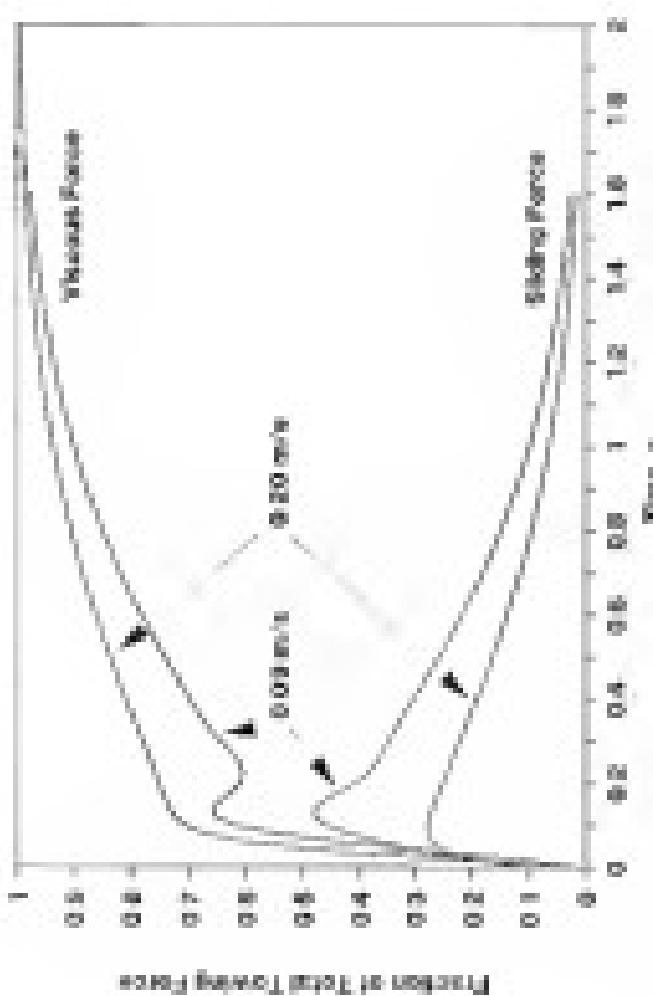
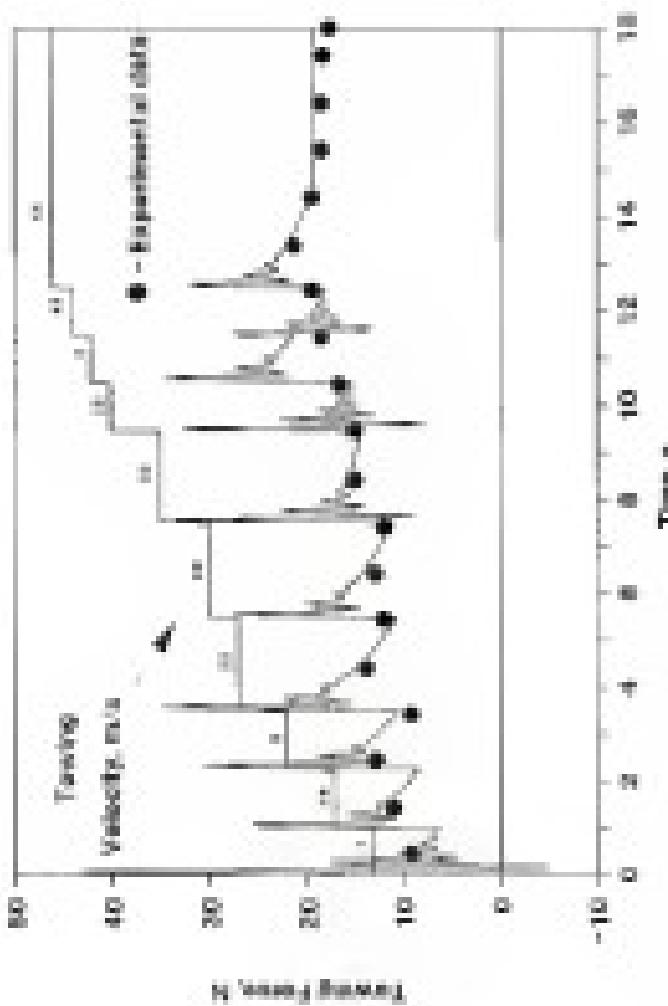


Figure 3. Effect of running velocity and time on grip force. The data measured 2 days on the bench and 1 day in the field. The magnitude of maximum grip force occurring during the run length.

Figure 3.1. Testing blood on a 2-DG treated mouse 1 h post-injection. The test indicates a significant increase in glucose uptake in the liver and heart.



over transient spikes. The latter is presented in Figure 5-4. Experimental data showed that the model accurately predicted the increase in tensile force with tensile velocity. The model predicted that over this velocity no increased, there would be a peak in the tensile force due to the inertia of the meadowlark device, and that the friction forces generated by the plant were not great enough to immediately affect the oscillatory response of the combined spring-mass system. However, these predicted peaks in the tensile force were not evident in the experimental data. This could have been due to the sparseness of experimental data in these regions. Also, the model used a step velocity increase, and it is possible that steps are nearly impossible to elicit.

On the other hand, we are surprised of larger plant friction forces large enough to dampen quickly the combined response of spring-mass components (Figure 5-8). Friction due to inertia of the plant was predicted by the model, but was not evident in the experimental data, although, the model accurately predicted the general increase in tensile force. This discrepancy probably was related to the practical impossibility of a step velocity input.

The advantage of reducing thermal effects through a slow increase in tensile velocity is that the maximum tensile force can be reduced by a factor of four (Figures 5-6 and 5-8). The "protein added" program can be used to predict the response of the species to other kinds of velocity inputs. Different step velocity inputs were simulated (Figure 5-7), and results indicate that thermal forces could be neutralized for small accelerations.

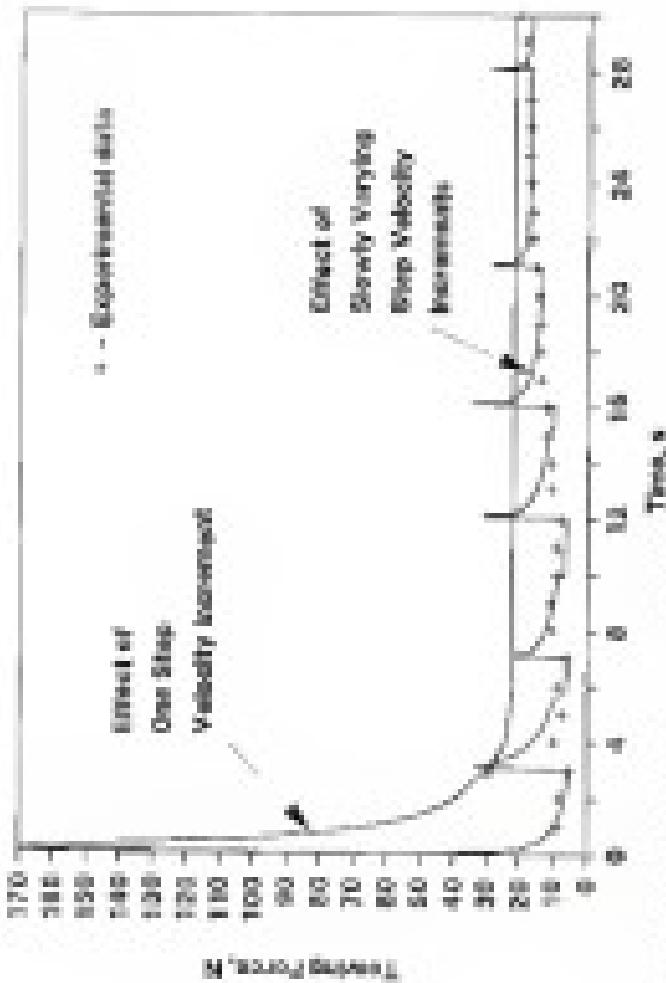
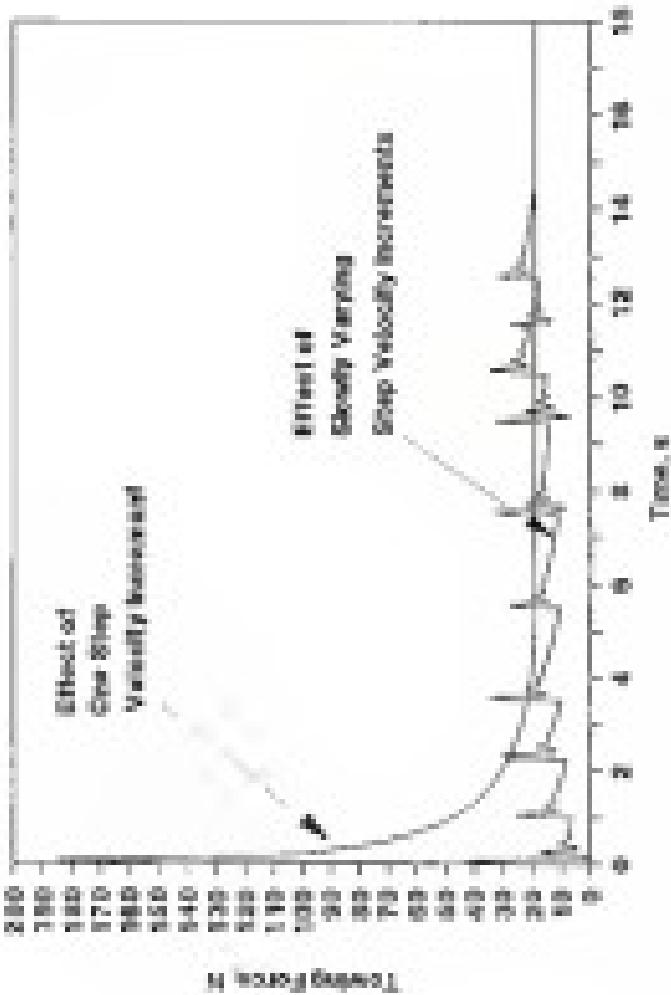


Figure 3.2 Half-response time (Response time = $\frac{1}{2} \ln(2) \times \text{Time}$) for slow, medium and fast vehicles in response to a one-step increase in velocity. The steady varying vehicle shows a large overshoot in response to the change in velocity.

Figure 3a. Influence of loading frequency on the effect of shear stresses on the shear modulus of the soil. The influence of the loading frequency on the shear modulus of the soil is shown in Figure 3a.



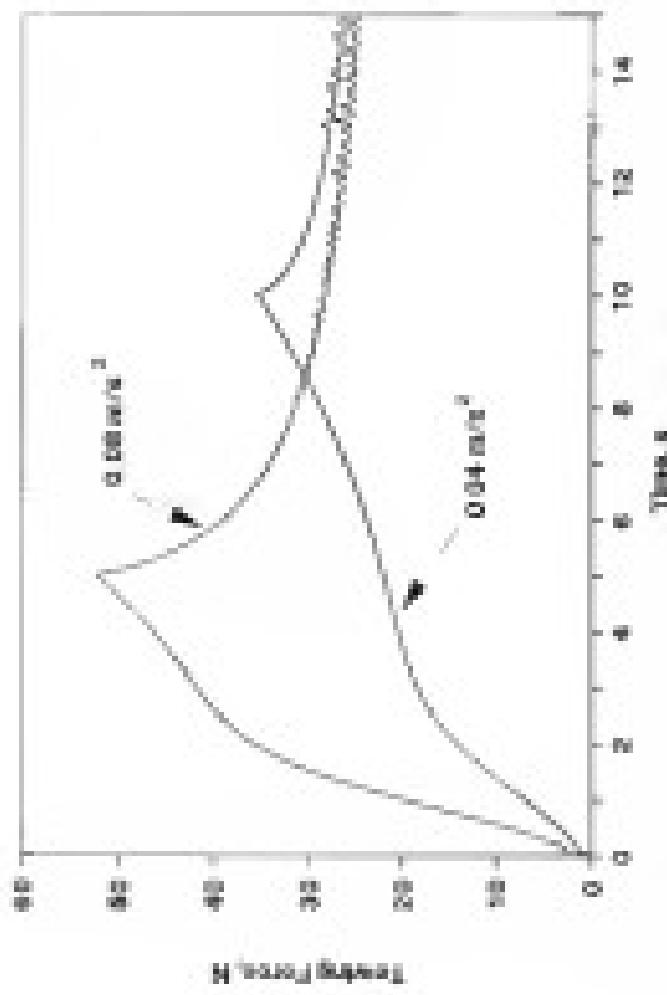


Figure 3.3 The mean response in terms of time spent in each state and average time spent in each state over time. Note that the two distributions are very similar, indicating that the mean time spent in each state is approximately equal to the average time spent in each state.

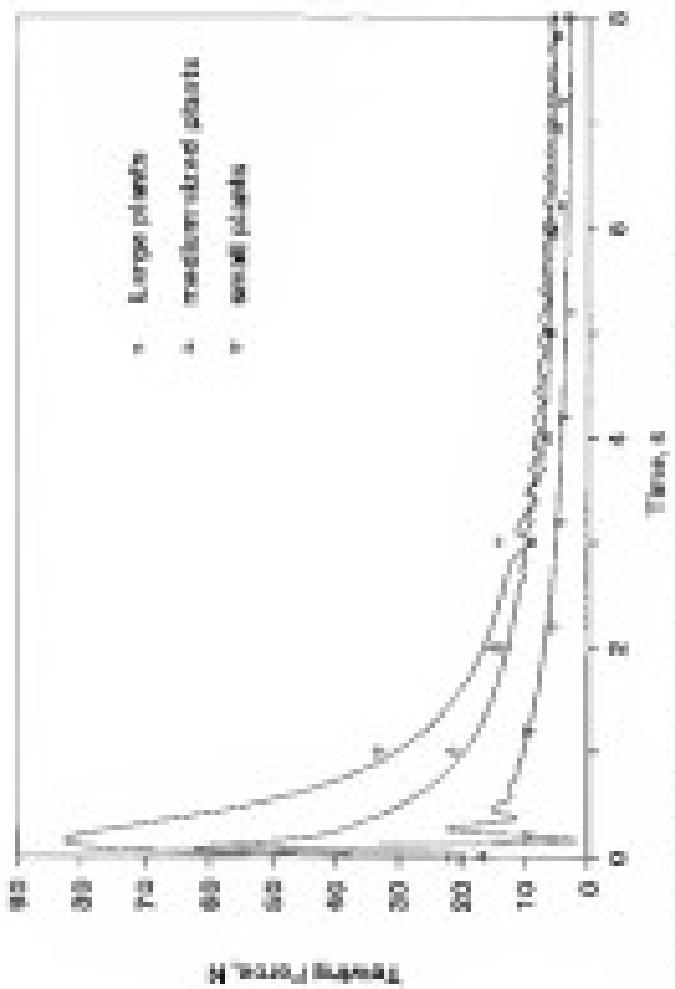


Figure 3A. Number of plants per square meter versus time. The curves start at 2 days at 100% germination and 100% survival.

Effect of soil desiccation. The effect plant size has on spring force is evident in Figure 6-8. Small plants (population density, 1000m^{-2}) have small rhizomes, and although the total number of plants is small in large, the total projected area is large. It would be expected in the expected total area from a set of larger plants. Therefore, as depicted in Figure 6-8, the rhizome force at the drying condition on small rhizomes was not large enough to dampen quickly the oscillatory response of the continued spring-force response. In contrast, oscillatory response was not evident in more compacted or large plants.

Also, initial oscillatory response of spring-force amplitude larger to damps [Figure 6-9] if mats are small. As the width of the mat decreased, oscillating force constituted a greater proportion of the total force in the spring and mat compaction (S) decreased [Figures 6-10 and 6-11; Table 6-2]. Oscillation of $\approx 1.00 \times$ large to $\approx 0.60 \times$ wide mat was approximately half the oscillation of $\approx 1.00 \times$ long by $1.00 \times$ wide mat. On the other hand, as the initial length of the mat decreased, mat compaction (S) increased slightly.

Table 6-2. Theoretical effects of mat size on desiccation. The mats were placed on approximately 0.1 m², and were comprised of randomly oriented plants weighing 0.1 g on to each.

Spring force L, R, V	Compaction S	Damping D ₁ (%)	Areal Stress kg/m ²
1.00 m x 0.60 m	1.00	21.2	
1.00 m x 1.00 m	21.70	36.0	
2.00 m x 1.00 m	5.00	36.0	

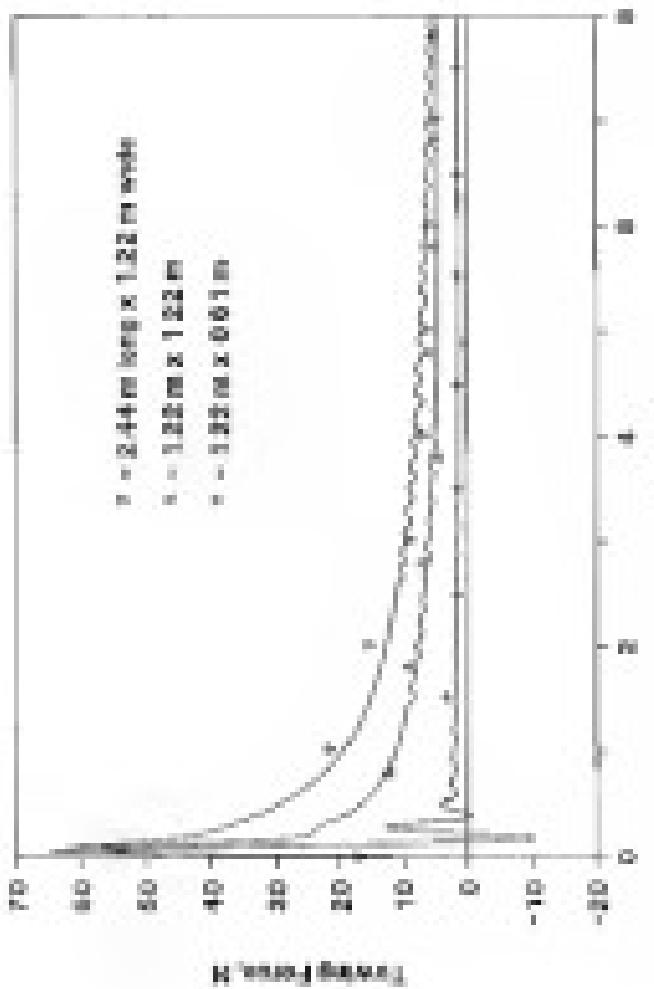


Figure 1. Average error versus number of trials. The solid line is for $\alpha = 0.001$, the dashed line for $\alpha = 0.005$ and the dotted line for $\alpha = 0.01$. The error is calculated as the standard deviation of the mean error over 100 runs.

Figure 10. Effect of β on the relative density distribution for various strain rates. The solid line represents the case for $\beta = 0.0$, while the dashed line represents the case for $\beta = 0.5$.

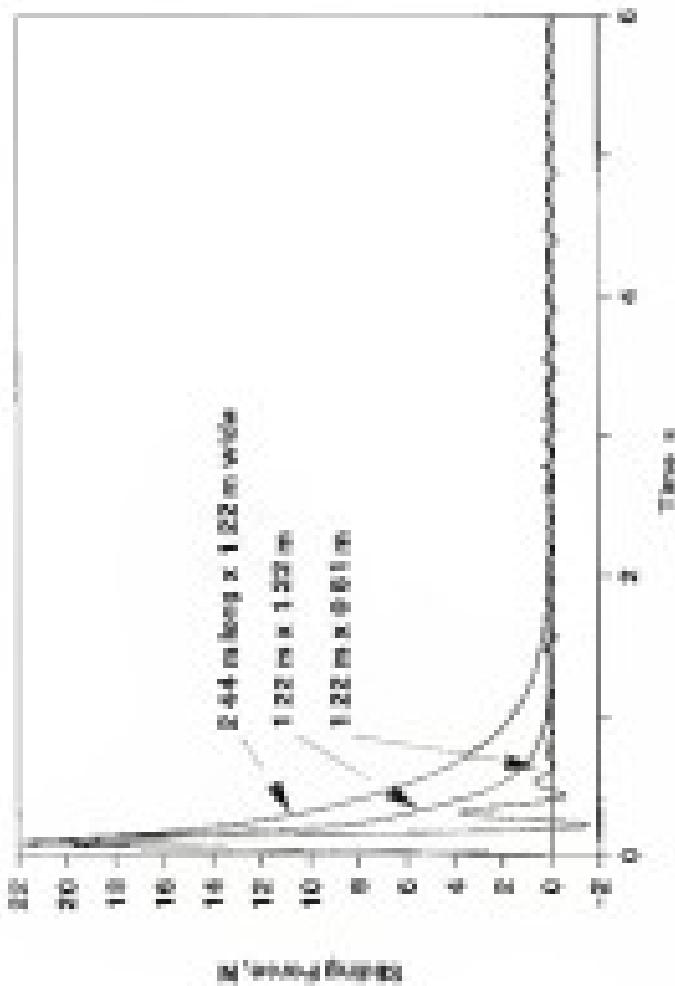
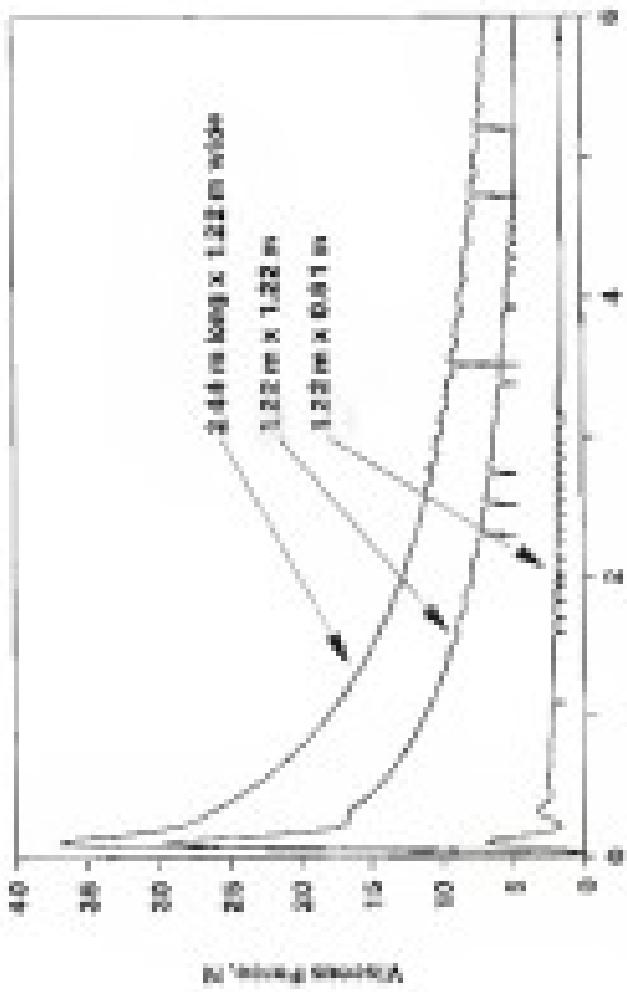


Figure 8. Stress-strain plots on different sizes of 3.15 wt% PVA films at a crosshead speed of 5 mm/min.



Bending moment of "pull under". Bending over through the centroid of a displaced volume and this centroid changes position if the body tips. Below a major ligament the reaction due to support by other plants or by hands. The centroid of displaced volume is positioned near the center of the object, and the center of gravity lies above the centroid of displaced volume. Thus additional induced gravity generates positive verticality [Hobson and Green, 1989]. Plants undergo little enough force to applied to overcome the buoyant force or if the plants are tipped so that their anisotropy lie below their center of gravity. Bending is defined as the summation of the buoyant force and unit line of the volume of the object.

The Bending Force of a set was described in chapter 3. Bending Force per unit area depends on plant mass density and on the planted condition of the plants. If plants are well connected, force above the normal buoyancy force is required to submerge a set due to the weight moment of a set. Force used in this study covered the range of weightless condition 0.0 m/s, although we are not able up to 1 m/s. This set was presented from computer, and its leading edge was not from the mouse. In general, the velocity when "pull under" occurred ranged between 0.4 and 2.0 m/s. Calculations indicated that force used in this study ranged from 123 to 300 Pa on submerged flow ($\rho g = 1 \text{ N/m}^3$) and required over 1000 N to submerge it.

Observations indicated that each comprised of different sized plants and connections became unstable at different levels of compression. For instance, a set consisting of small plants became unstable when compression was approximately 50 N, and ones consisting of larger plants became unstable when compression was approximately

120. These compaction data were independent of sample size if the soil was composed of plants at the same site and same density.

One of the assumptions made in the development of the model was that a soil responds until the stresses reach (Oedometer 4). This requires compaction of approximately 60%. After observing that non-normally "well graded" or loose compaction, the model was changed in order for the behavior of bridges to match [i.e., enough resistance must be sustained within the soil to prevent further compaction]. To determine the system response when resistance is present, master elements are added to the conceptual model representing the elastic nature of resistance to excess. It is described as follows:

$$\text{Master response of resistance} = R_p \left[1 + (R_s - R_p) \cdot e^{-R_s} \right] \quad (5.17)$$

where R_p equals the unloading response of a given soil, and $R_s = 10 \times 10^{10}$ Pa. The constant denoted R_s is the elastic modulus of resistance calculated in Chapter 4. This element was positioned in parallel with the constant resistance element. With the addition of this element, Equations 5.11 and 5.12 changed to:

$$\frac{\partial u}{\partial t} = (K_s \ln(1+1000 R_p R_1 R_2 T_1 T_2) - R_s (V_1 T_0)) \\ - R_1 (L + L R_1 + R_2) \cdot \frac{R_1}{R_2} \cdot \frac{R_2}{R_1} \quad (5.18)$$

$$\frac{\partial u}{\partial t} = (R_1 (V_1 - T_0) - K_p (L + (R_1 + R_2) \cdot \frac{R_1}{R_2} \cdot \frac{R_2}{R_1})) \Delta t_n \quad (5.19)$$

These expressions were tested by the "problem solver" program. Results showed that this expression equaled R_p , owing forces per

unit net area extended 200 Pa. Thus, from the definition of cohesion, we provided that the plane contact at an angle and a vertical elastic contact force is produced. This is illustrated in Figure 4(a). Observations showed that planes were tilted and moving at "full scale". Real output legal "full scale" was modified according to *because* the program solves only one-dimensional problems. Furthermore, calculated testing velocity at "full scale" were within 10% of the experimental values.

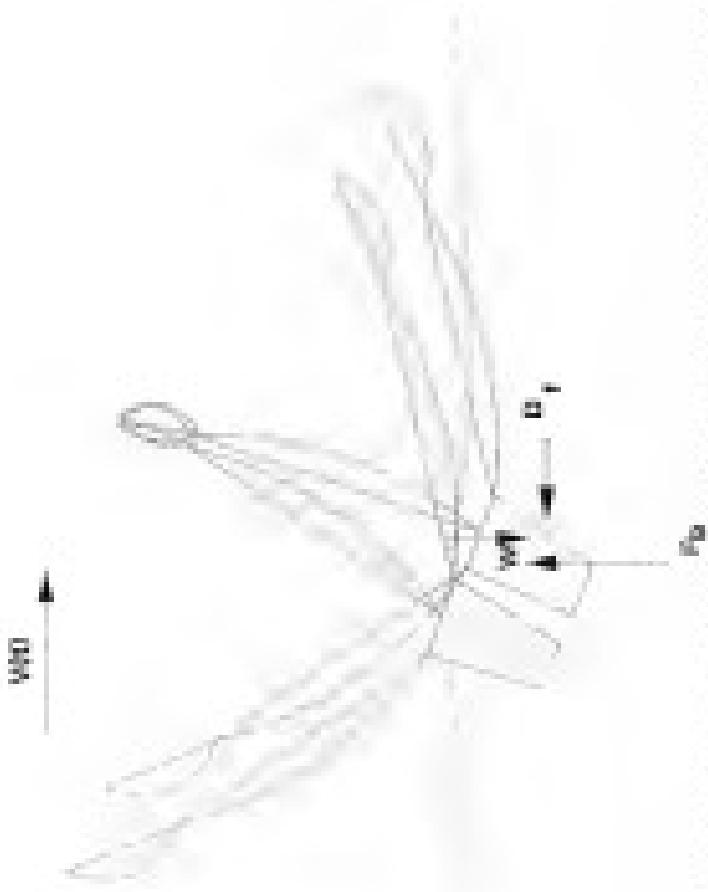
Results

All of major impacts used within a rectangular enclosure were modelled. The input consisted of one material element, one for the soil and one for the enclosure, a common friction element, a viscous friction element, an adhesion element for the enclosure, and an adhesion element for enclosure contact.

All of the parameters required to solve the model were estimated from comparison of experimental and computer simulation data.

- The model successfully predicted testing force and displacement for different sizes and types of nets and velocity inputs.
- It failed to predict shear and forces which are presented

Figure 3.11 Extended representation of a plane parallel to "Null sheet". The shaded plane contains \mathbf{B}_0 , oriented such that the standard basis vector \mathbf{z} lies in its plane. A second plane, oriented such that its normal vector is \mathbf{n} , lies in the same plane. However, as \mathbf{B}_0 is in the same plane as \mathbf{n} , the angle between them is 0° . This means that the magnetic field is parallel to the direction of current flow in the plane of the figure.



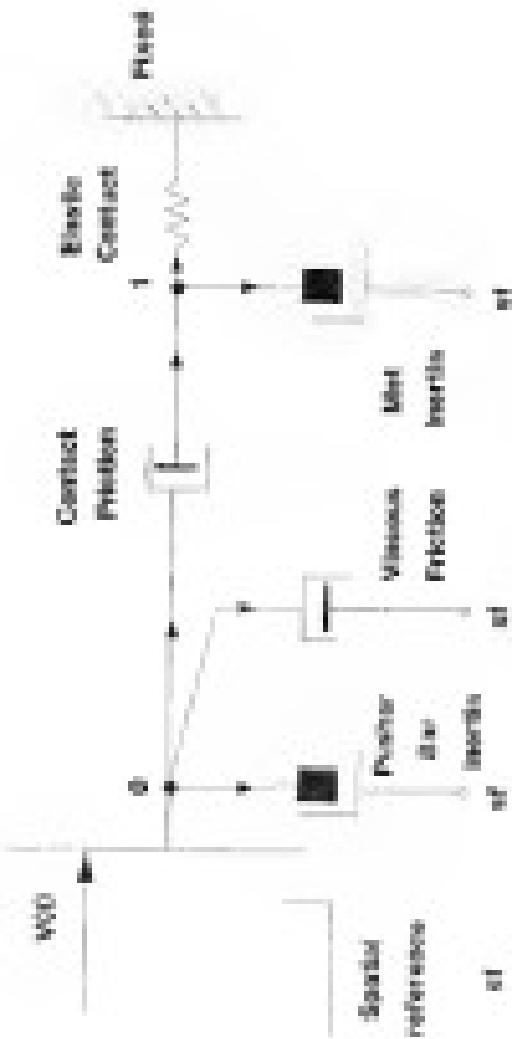
CHAPTER VI NET COMPRESSION MODEL

The model of a wire spring test presented in Chapter 5 was used to determine the response of a net constrained at one end to compression from the opposite end. Calculated compressive force was compared with experimental data. This information can be used to determine the capacity of spring devices and to design more efficient spring devices.

Constrained Model

The simplified model of a wire spring test of length L and width B under a compressive force is pictured in Figure 6-8. In this model, the size components developed in Chapter 5 are used to represent the wire spring test. The compressive force represents the test equipment used to constrain nets. As data obtained from experiments would be used to validate the net model. The net was clamped between two bars, a fixed poster bar and a fixed rear bar. The front bar holding the net was considered rigid; therefore, only the net is represented. The back bar, or the other front bar, is a similar passive element intended to be added to represent interaction between different tail the rear bar.

Initially, the system is at rest. The distance from node 0 to node 1 is the rest length of the net, or the front bar stress, or



Mechanical System Compressive Forces

Figure 4.3 Decomposition of contact forces subject to compression.

velocity V_0 , (1) represents the net against the back bar, and the net shear stress. Comparison conditions with the plane "wall under" body formulation.

Body formulation.

The element descriptions were given in Chapter 8. Recalling that for applied at node 1 produces the following equation:

$$T_{R_0} = T_0 + T_{R_1} = 0 \quad (8-0)$$

then

$$T_{R_0} = R_0 \frac{V_0 - V_1}{L} \quad (8-1)$$

$$T_0 = R_0 (V_0 - V_1) \quad (8-2)$$

and

$$R_{R_1} = R_1 (X_1 - L) \quad (8-3)$$

From R_{R_1} is the elastic-energy loss of plane against the fixed bar, and is equal to that $R_1 = L$. Free length of the net is L , and R_1 is deflection of spring at node 1. Elastic modulus $E_p = 2 \times 10^7$ N/m which equals approximately the inverse of the angular modulus of elasticity of the two connecting bodies (ribbons and wind over bar) subjected to net strain ϵ . The mass variable equation are:

$$\frac{d^2\epsilon}{dx^2} = V_1 \quad (8-4)$$

$$\frac{d^2\epsilon}{dx^2} = (R_1 (V_0 - V_1) + R_0 (R_0 + L)) / R_0 \quad (8-5)$$

The output equations were chosen to describe net compression and total compressive force. Compressive Force was calculated as given in Table 2 from input variables and computed mean variables for class A [2]. As:

Compressive Force =

$$= R_{\text{c}_0} \int_{0}^{R_0} r^2 \pi \times 1000 \rho_p A_p C_p R_0^2 + R_c (T_0 - T_0) \quad (2.1)$$

where

$$\frac{\int_{0}^{R_0} r^2 \pi}{R_0^2} = \text{constant for a given density layer}$$

Net compressive was calculated on the basis of the class A specific area, because specific area compressive force difference is related to initial yield starting densities of each specific area. Specific area A_p is defined by:

$$A_p = A_0 / R_p \quad (2.2)$$

where R_p equal the sum of the plastic within the frame and A_0 again the sum of the the compressive force of class A

$$A_0 = (k - k_0) \cdot l$$

where k_0 was obtained after the integration of $T_0 = \frac{\partial \sigma_0}{\partial r}$

Method of Selection and Parameter Estimation.

Equations (2.1) and (2.2) represent a system of coupled three-dimensional differential equations. Together with the initial conditions they represent a problem of initial hydrodynamics. The equations were solved to determine the behavior of a soil subjected to different compressive velocities, modulus of "solid state", and parameters R_p .

and C_2 of steady flow condition. The former was studied because their descriptions were not presented equal to those for solid trees.

To calculate the resistance potential and to solve the system equations, the fourth-order Runge-Kutta formula was used, and a positive value range was within the calculated values of the compressive and compressive force every 0.002 s (Appendix 1). Initial conditions were $V_1 = 0$, $R_{11} = 0$ and $R_{21} = 0$. Input velocity was a function of time, and it was specified to be a step to the first 0.2 s and a constant after $t = 0.2$ s. Input descriptions were

Type I: Acceleration

$$V(t) = \frac{V_0}{\theta \beta} t \quad \text{for } 0 \leq t \leq 0.2 \text{ s and}$$

$$V(t) = V_0 \quad \text{for } t > 0.2 \text{ s}$$

Type II: Acceleration

$$V(t) = \frac{V_0}{\theta \beta} t \quad \text{for } 0 \leq t \leq 0.2 \text{ s and}$$

$$V(t) = V_0 \quad \text{for } t > 0.2 \text{ s}$$

where V_0 was the steady compressive velocity. These velocity terms were chosen because they reflected accurately the experimental compressive velocity. For the experiment, however, input 0 was necessary not used to attempt to produce a better correlation between experimental and calculated compressive force.

Parameters A_2 and C_2 for tensile were obtained in Chapter 8; however, the boundary conditions of a tensile are different from those in compression. In compression, mass was desired to compact. Compressive stress changes greatly and, therefore, uniaxial friction coefficient B_2 should be. Since drag coefficient α is not directly measured, tensile load can be the same as the compression. Therefore, parameters C_2 and B_2 were estimated for compression. The "problem solver" accepted estimates for these parameters, solved the equations and reported values of tensile force and the compression as functions of time. The compressive force was compared with experimental values. This procedure for parameter estimation was repeated until acceptable correlation ($r^2 > 0.90$) between calculated and experimental data of force for different particle size and different compaction velocity was achieved.

Initial estimations of parameters A_2 and B_2 were based on some prior information. Lettability B_2 was held constant since it was found to be a constant in Chapter 8 and A_2 was assumed to vary as α in compression. Because drag coefficients for steady flow conditions drag coefficient was determined to be a function of soil geometry [Equation 4-18] and because the distinct characteristics of a soil in compaction is reduced soil length, C_2 was initially varied according to a variation of Equation 4-18

$$C_2 = 0.4920 \exp(0.006 - \frac{L}{L_{max}}) \quad [4-2]$$

Downward Counting Data

A trap rig was designed and constructed for this investigation. The 3 m by 3 m enclosure frame (Figure 8-1) consisted of a wire moving along a pair of vertical posts and a similar fixed side. Below this on 30 cm centers extended 30 cm below the side rods and 30 cm above. The moving rods weighed 22.3 kg., and 11 m was driven by electric or hand wind which, in turn, were driven by a hydraulic motor or a modified automotive motor. With these drives four different constant separation velocities could be obtained approximately 0.05 m/s, 0.10 m/s, 0.15 m/s and 0.2 m/s.

Four solid surfaces were transversal from the rods in the rods through a wire support carriage equipped with lead cells to measure force and parallel hydrogen. Signals from the lead cells and free positions moving plates however were recorded on a Gagecraft Scientific CRT data logger equipped with "Burst" mode. A separate switch allowed sampling intervals of 0.01 s.

Data were collected on plants at the following locations— 1760 State Laboratory, Between, HI; DAS Seine Research Unit, HI; and Sea Disney World Disney plant, Kissimmee, FL. After the site was selected (Figure 8-2), seven plots were characterized by plant height, root length, root, petiole area, number of peduncles and total length. All plants inside of the traps were weighed, and the weight was divided by the area of the frame to yield an average shooting area densities. Between length and population density were calculated using the expression presented in Chapter 3. Root biomass and density were measured in some experiments, and corrected for others.

Figure A.7 Incorporation occurs and suspended sediment within the bar and back-tube deposited at both ends of the bar. As the bar moves toward the front end however, the sand compacted.



Figure 6.1 A never-before-seen new form, conceptual

Figure 6.2 A never-before-seen new form, empirical



Compressions were performed on 16 different soils and characteristics of these soil shear tests are presented in Table 4-2. Sixty soils were compared to test the soil under different conditions of compaction, velocity, consistency, sand density, plant size and orientation, and to test the equipment. These 60 soils were sufficient for parameter extraction.

Experiment	Table 4-2. Characteristics of soils tested			Soil	Observation
	R_p	θ_p	θ_1		
	kg	m ³		Length, m	
1	1800	300 Ø	0.000	0.80	well rounded
2	1700	150 1	0.000	0.20	
3	1800	150 Ø	0.000	0.20	open spaces
4	1800	170 Ø	0.0000	0.20	
5	1800	150 Ø	0.000	0.20	sticky

Before a compaction experiment started, a grid of rulers was placed on top of the soil. The grid was used to determine when "troll under" occurred because it begins when grid lines touch closely together. All experiments were videotaped. After the tapes were viewed, the position of the setting ruler at "troll under" was recorded.

Results and Discussion

Experimental Observations. Reported observations pertain to force compaction behavior, "troll under" and blowcounts. Compaction experiments indicated that as compaction velocity increased, the relationship between compaction force and compaction changed. At low velocity ($< 0.30 \text{ m/s}$) the relation between compaction force and specific area was approximately linear up to "troll under". At higher velocity ($> 0.30 \text{ m/s}$) the relation was obviously nonlinear. The

positions within a unit share building or "wall side" material appears to be the most dense form of the unit, and therefore, is measured at different locations within different units. Generally, if plates were well dimensioned and built up to the effect of "wall side" material at the same speed. In contrast, earlier experiments showed that plates are tapered from the rear of a unit when the set started from rest.

Frictional-dissipation: Parameters C_0 and R_0 varied as a set response time. R_0 decreased with increasing separation velocity and consistency. The drag coefficient followed the relation given by Equation 8.8, which was further evidence supporting the unit drag coefficient developed in Chapter 4.2.

On the other hand, R_0 was found not to be a constant, as it was in the testing model, but to increase with displacement of the plates here. Friction coefficients were proportional with velocity now (Vitose and Frederick, 1979). Since water frictional constants were increased with water flow displacement and plate separation, the following equation was deduced:

$$R_0 = R_{00} S_p K_p \quad (8.9)$$

where R_{00} is a constant. After this expression was substituted into the model, calculated and experimental tangential force agreed (Table 8.1). Correlation coefficients ranged from 0.96 to 0.98. The one significant shortcoming was not strong ($r^2 = 0.93$) will be discussed later.

Table 6-1. Coefficient friction and Froude and other variables used in the simulations of soil compaction.

Exp.	Compaction velocity m/s	R_s	Ball diam. mm	Froude no. 1	Correlation between measured and model r^2
				soil compaction 2	soil compaction 3
4	0.05	11745	34	1	0.88
21	0.10	7700	36	1	0.91
2	0.05	6500	34	1	0.88
22	0.05	3500	34	0	0.93
1	0.05	3500	32	1	0.89
23	0.05	3500	34	1	0.91

¹ See text for explanation.

Fricition coefficient, R_s , decreased with compaction velocity and consistency (Table 6-1). Points in experiments 3 and 6 were all low consistency, and points in experiments 1, 2 and 4 were highly and highly consistent. Friction coefficient of all three soils compacted at the same velocity was always significantly lower because of their consistency. Friction coefficient of well-estimated soils decreased linearly with compaction velocity, and the regression relation follows ($r^2 = 0.88$)

$$\frac{R_s}{R_{s0}} = 0.9415 - 0.1333 \cdot V_0$$

The four data points were omitted as slopes a regression equation for soils of low consistency; however, R_s of these were ranged from 3500 to 6500 of those of highly consistent soils. Also, for comparison,

initial velocity was 0.35 m/s. R_1 represented the value obtained from control experiments.

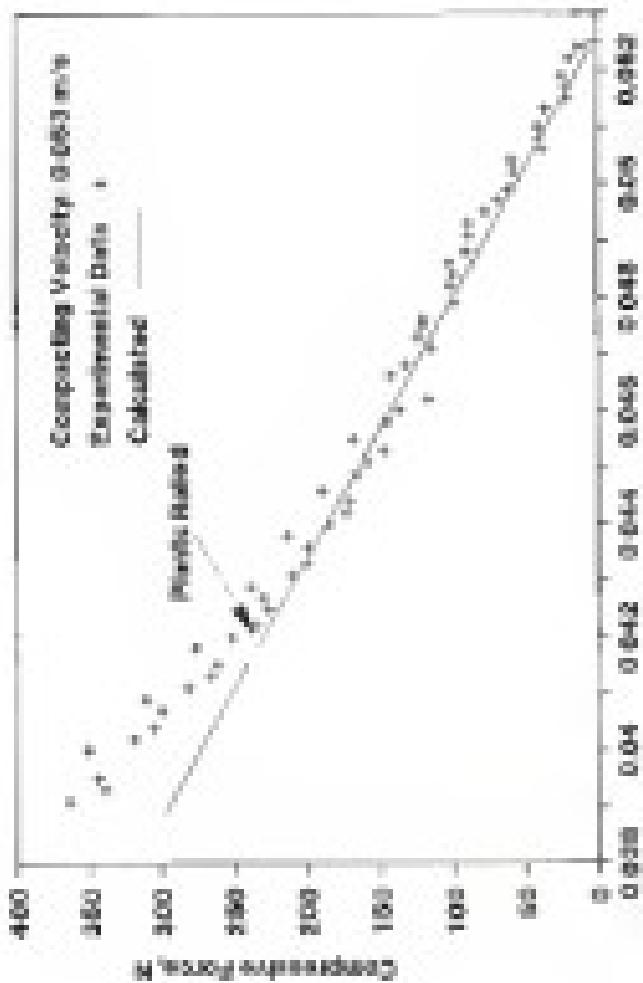
An explanation for R_1 as a function of velocity can be found in Chapter 6, and suggests increased tractive force with testing velocity until "full adhesion". This implies that a car reaches a traction coefficient and "full adhesion" without need for a rear resonance if the velocity is high enough. During suspension experiments, however, "full adhesion" is a condition satisfied at lower suspension velocity by restraining it and forcing it to respond. Therefore, more deflection force is required as the suspension load limit the response is not until "full adhesion" and is reflected in a larger friction coefficient.

Brake behavior and reduction of "full adhesion". At low suspension velocity, the fraction of total compressive forces due to contact friction was greater at higher velocities, as was used. Groups of experienced drivers showed that as the suspension velocity, the relation between compressive force and specific grip area (Figures 6-3 and 6-4) was linear. Calculated drag indicated that a driver can decelerate at velocity below 0.35 m/s, but is beyond adhesion very quickly above 0.35 m/s. As the same time, greater forces happens from grip loss (Figures 6-8 + 6-9).

In general, compressive force increases as suspension velocity increases (Figure 6-6). The model could not successfully predict compressive force to a test that accelerated initially at low velocities (Figure 6-6). Oscillatory behavior seemed to be more in these cases. Furthermore, inertia of the system led caused a rapid increase in compressive forces at the beginning of compression (Figures 6-8 and

El análisis muestra que el efecto de la variación en la tasa de crecimiento es más fuerte en las economías con menor desarrollo tecnológico y menor diversificación industrial.

Figura 10.4. *Efectos de la tasa de crecimiento*



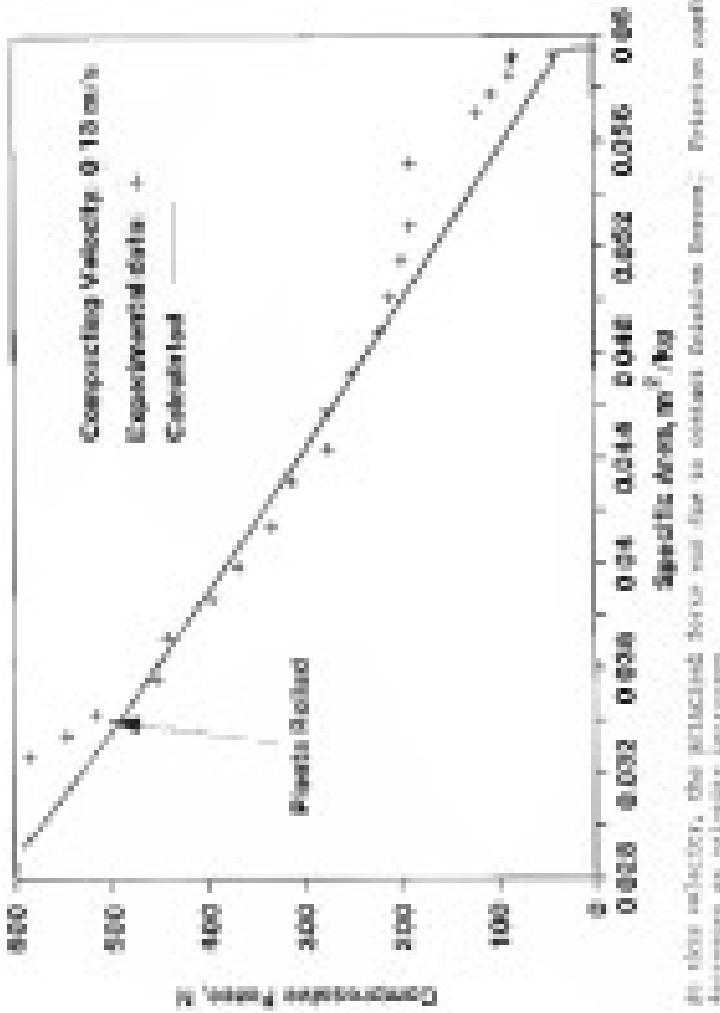
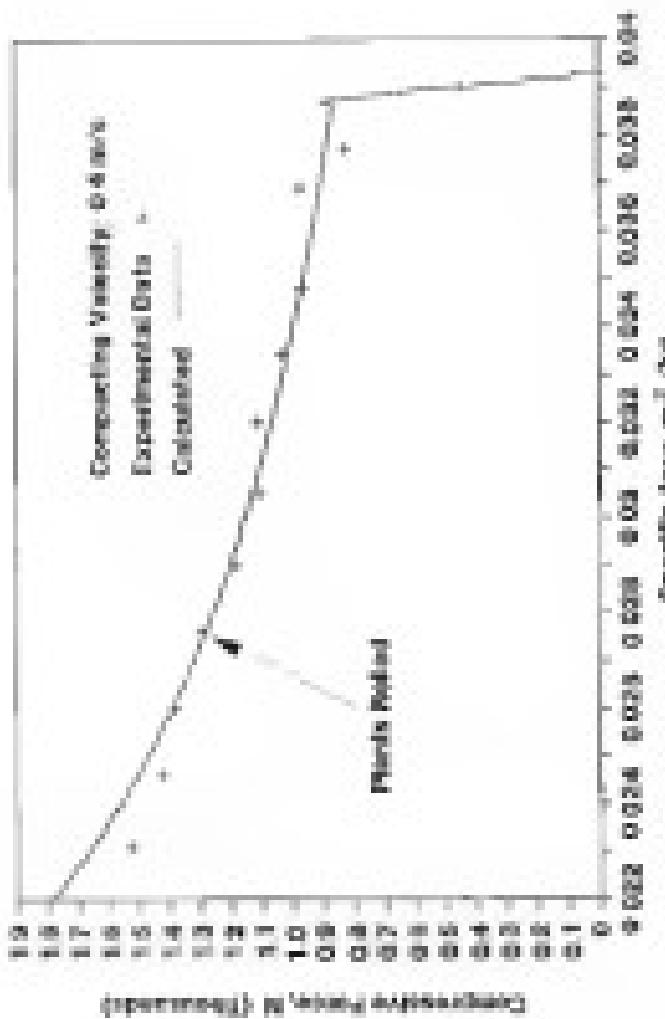


Figure 1. In this figure, the measured data are shown to obtain polarization current decreases as solute increases.

Figure 6.3. *Estimated results of the simulation study. The influence of the ratio of the number of individuals to the number of households on the mean relative prevalence of the disease.*



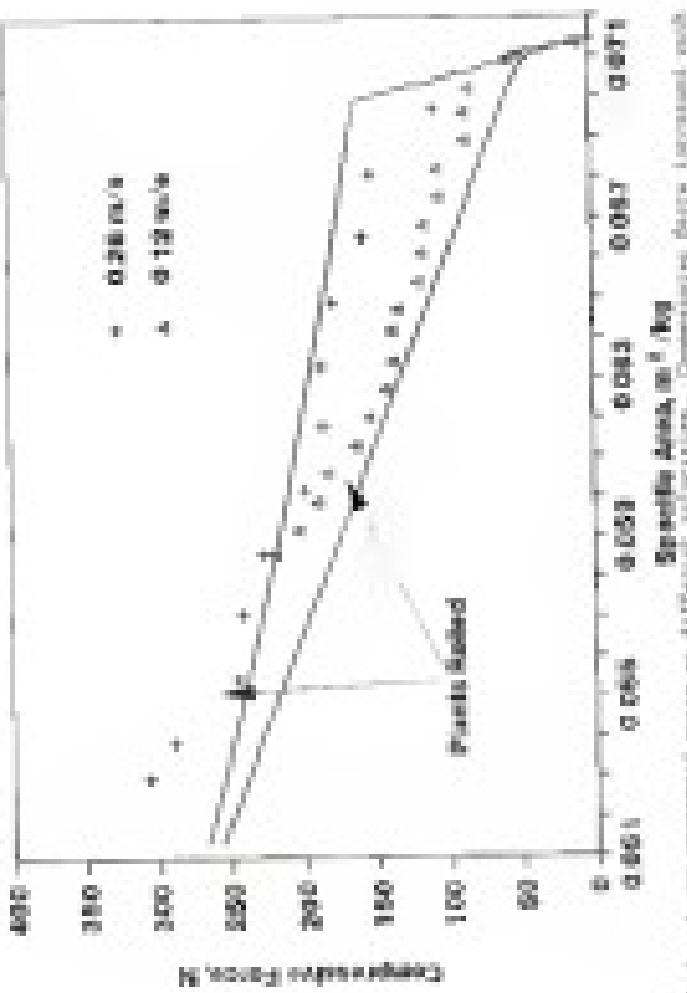
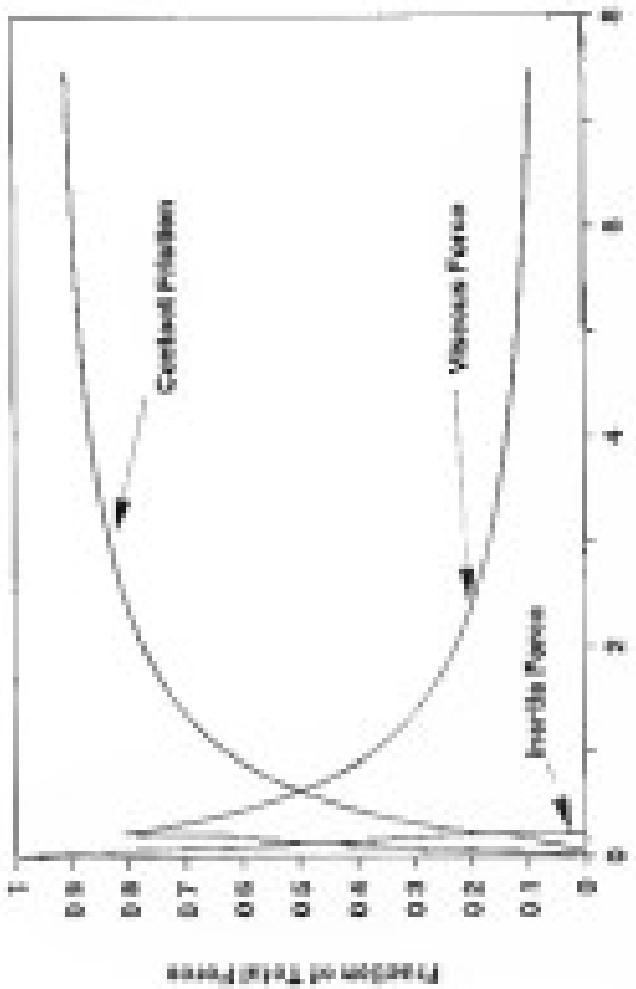


Figure 1. A) Individual cluster size distribution. Distribution of cluster sizes in the population with respect to the

Figure 4. Effect of different types of organic manure on growth and yield of maize at two locations in 1993.



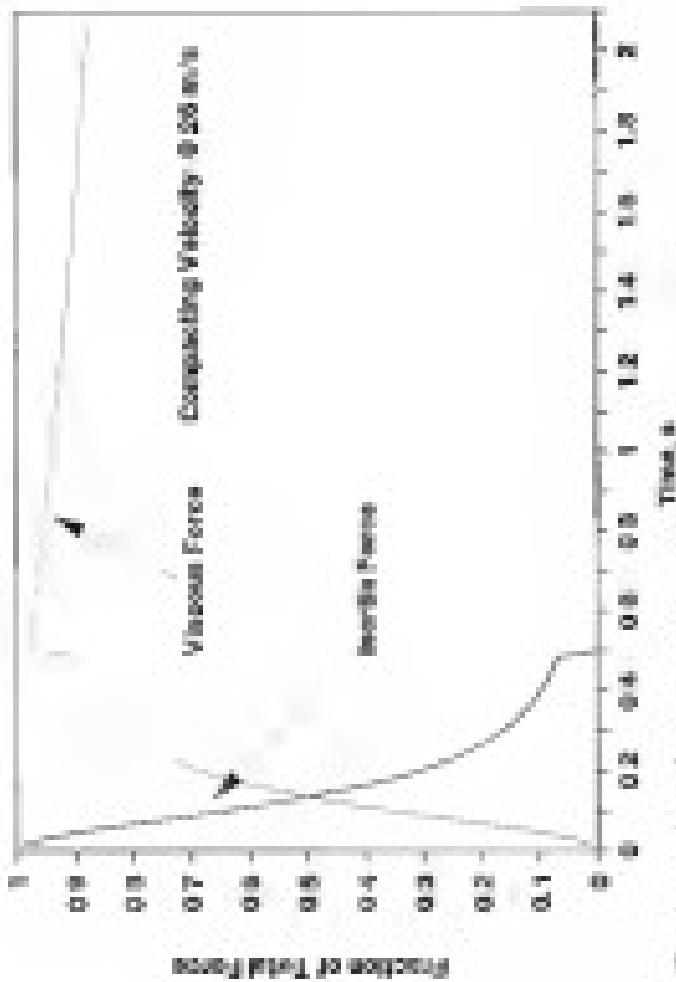


Figure 1. The competing velocity has higher than Type A, independent linear was due primarily to nonlinear function (dotted), linear was due to the competition for a single resource.

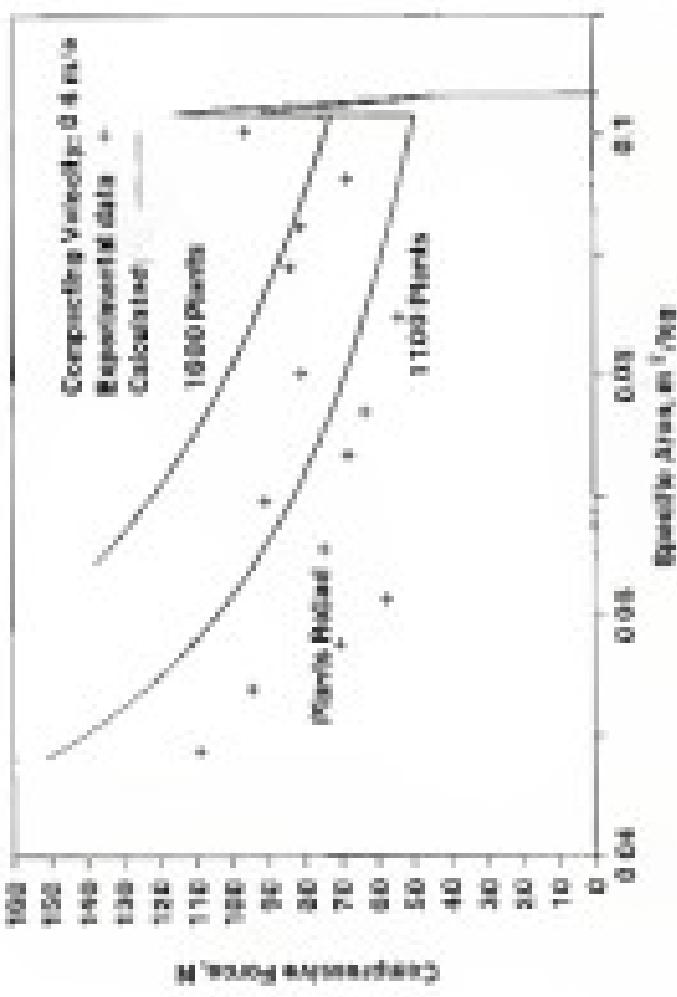


Figure 1.1 The net leverage of the NYSE versus Nasdaq. Companies listed on the Nasdaq have more debt than those listed on the NYSE. This is consistent with the market's view that Nasdaq firms are riskier than NYSE firms.

(δ E), became the main element representing the sum of the compressive force on the principal Miller component of the system.

Computation (3) of "full value" varied according to plant population and plant length. Generally, it decreased as population increased and average plant length decreased. Plant length was not a factor after all plants tested had more roots. "Full value" ranged between 30 and 100 percent, which are higher than the values in testing cases. "Full value" remained at the front and off center during testing cases but inside the set during computation cases. Therefore, plants were generally supportive in suspension, adding to more difficult the task to move the car. Compressive force on a set of large plants was large enough to damage the set, provided the plants were tipped to obtain the required reaction force component. Damage will occur at large plants. "Full value" was usually accompanied by a large increase in compressive force.

Results

A mathematical model was developed to predict the response of a four wheeled vehicle to lateral compression. Individual column parameters, C_0 and R_0 , were estimated through comparison of calculated and experimental compressive forces. Friction and lateral varied with compression velocity, temperature, and material type. These relationships varied with the change in length of n and m .

The model for soil suspensions adequately predicted compressive forces. "Full value" can be predicted as a basis of suspensions or suspensions.

Compressive forces are dominant for drag < velocity exceeding 0.30 m/s and by passive force at velocity below 0.10 m/s.

- Cooperative form increased with competition intensity
- Cooperation (Z) or "trill value" ranged from 20 to 400, and increased with increasing plant population and decreasing average plant length. Z is not affected by competition intensity

CHAPTER 8.1 CONCLUSIONS AND RECOMMENDATIONS FOR FUTURE RESEARCH

Better hypotheses were more described quantitatively in terms of plant mass, plant density, soil texture, compaction, and plant length. These physical characteristics were parameters in hydromechanical models describing (1) soil loading and (2) soil induced compaction. Critical physical properties of the soil include soil texture, average plant length, standing density, and average plant mass.

To develop any hydromechanical model, a relation between drug load velocity is required. Rover's drug load can used to describe this relationship for $\alpha = \infty$. In this case, area in the direction of flow is projected area of the total number of ridges. Drug coefficient is independent of Reynolds number, and it is related nonlinearly to soil geometry. A higher drug coefficient is associated with higher soil-to-length ratio.

In our experiments (i) the load on ridged upland is larger generally. For compaction between 30 and 40% (compacted length/L = 100) ridges imposed higher- and greater forces are generated. By the use of large deformation theory, shear properties of ridges were

absorbed. Pollock's ratio and Young's modulus were determined, respectively, to be 0.47 and 10.0 GPa.

Conventional models were developed to describe nuclear reactor hydraulic interactions for two different freezing operations: rectangular and trapezoidal (fixed) compaction. Basic and characteristic elements of two additional particle contact friction elements, mass element of inertia, and viscous shear and viscous damping components elements were incorporated all three available nuclear hydraulic systems. Their descriptions and relationships were based on experimental observations and physical and hydromechanical properties. The equations derived from the empirical model were solved numerically using a Runge-Kutta integration method. Velocity, pressure, temperature, force and heat dissipation were predicted over time with good correlation with experimental data. The instability or "mill末期" was found to occur at a specific low compaction.

As a tool for the design of better harnesses, the model could be used to determine:

- Optimal tractive and compaction conditions for different freezing schemes and
- Effect of different acceleration profiles
- Power and energy requirements of different designs

The design of equipment, such as storage tanks, valves, control devices, and monitoring devices, could be based on the following hydromechanical information from this research:

- Choosing accelerating a car and moving machine parts to reach a constant velocity uses energy. Because the highest forces in

the system related were due to aircraft, plane should be kept moving at constant velocity.

- Drag Force is reduced by using long narrow wings.

- "Ball under" can prevent by introducing appropriate planform or tail operating speed.

- Delaying or not to decrease the drag coefficient could save fueling costs.

Additional research should be done for large scale verification and application of the model. Different aircraft parameters concerning fuel quantity, flight altitude and distance, could be investigated to determine the relationship between the two parameters. Dependency of this relationship on other plane characteristics could be identified. For this, however, C_d was determined only for rectangular wings, drag coefficient could be determined for other wing geometries.

Furthermore, development of the program where "there is uniform progress with logic and target success" is needed for user convenience.

Finally, the present study of is not sufficient solving performance investigation. Study of more than under a moving test, which is a more complicated problem, could describe not enough the its negative consequences.

Appendix B
Problem Solver Progress

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Editorial and Production Team | Page 3

1

Page 1

- 1 Prevalence estimates of the glaucoma, as measured with visual field tests. These
2 were used to estimate the prevalence of glaucoma.
3 The Program using equation 100-1 would provide a
4 prevalence estimate based on a reference test study (assumed).
5

Journal of Economic Surveys (2018) 32:26–103

Journal of Space Weathering & Climate Vol. 10

Appendix B:
Executive Briefing Program
For Competition Characterization

University of Minnesota, Twin Cities, Minneapolis

Journal of Statistical Theory and Applications (2021)

Volume 10 Number 10 October 1998 ISSN 1062-1024

University of California, San Francisco - The Warren Alpert Medical School

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BIOGRAPHICAL NOTES

Reagan Jane Petrelli was born in Grand Rapids, North Dakota. She spent most of her primary school in Berthold, Minnesota. There she graduated from high school in 1953. At the University of Minnesota, Duluth, she received in 1959 a B.S. degree in Mining. The Pennsylvania State University awarded her a M.S. degree in agricultural engineering in 1971.

Before she started graduate studies at the University of Florida, she worked for several years in Texas. There, she worked as project in the areas of physical properties, process engineering and development, and tool engineering. Her basic interest is applying science to an alternative to conventional energy sources. It was this interest that brought her to the University of Florida for graduate studies.

I certify that I have read this study and that to my opinion it conforms to acceptable standards of scholarly presentation and is fully adequate, in scope and quality, as a dissertation for the degree of Doctor of Philosophy


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